





project capable of transporting similar volumes of natural gas may result in the expansion of existing natural gas transportation systems or the construction of new infrastructure; both of which are likely to result in impacts comparable to those described in section 4.0 of this EIS, we conclude that in addition to not meeting the Project objective, the No Action Alternative is also not likely to provide a significant environmental advantage. Therefore, we dismiss it from further consideration.

3.3 SYSTEM ALTERNATIVES

System alternatives to the proposed action would make use of existing natural gas transmission systems/facilities to meet the stated purpose of the Project. Implementing a system alternative would make it unnecessary to construct all or part of the Project, although some modifications or additions to an existing transmission system may be necessary. Existing pipeline systems and systems under construction are depicted on figure 3.3-1.

3.3.1 Existing and Approved Natural Gas Pipeline Systems

There are currently two existing FERC-jurisdictional natural gas pipeline transportation systems operating near the Project area: Transcontinental Gas Pipe Line Company LLC (Transco) and East Tennessee. There is also one approved FERC-jurisdictional natural gas pipeline system, the Atlantic Coast Pipeline (ACP) Project that is currently under construction. It consists of 604 miles of natural gas pipeline in West Virginia, Virginia, and North Carolina. The ACP Project is approximately 100 miles east of the proposed Project Dan River and Haw River interconnects. Additionally, one non-jurisdictional pipeline system owned by Cardinal Pipeline Company, LLC (Cardinal Pipeline) is operating near the Project. Without modifications, these pipeline systems currently do not have the available individual capacity, combined available capacity, nor direct physical connection to transport the required volumes of natural gas to the delivery points proposed for the Project. Therefore, we do not consider use of existing pipeline systems as a technically feasible alternative to the Project.

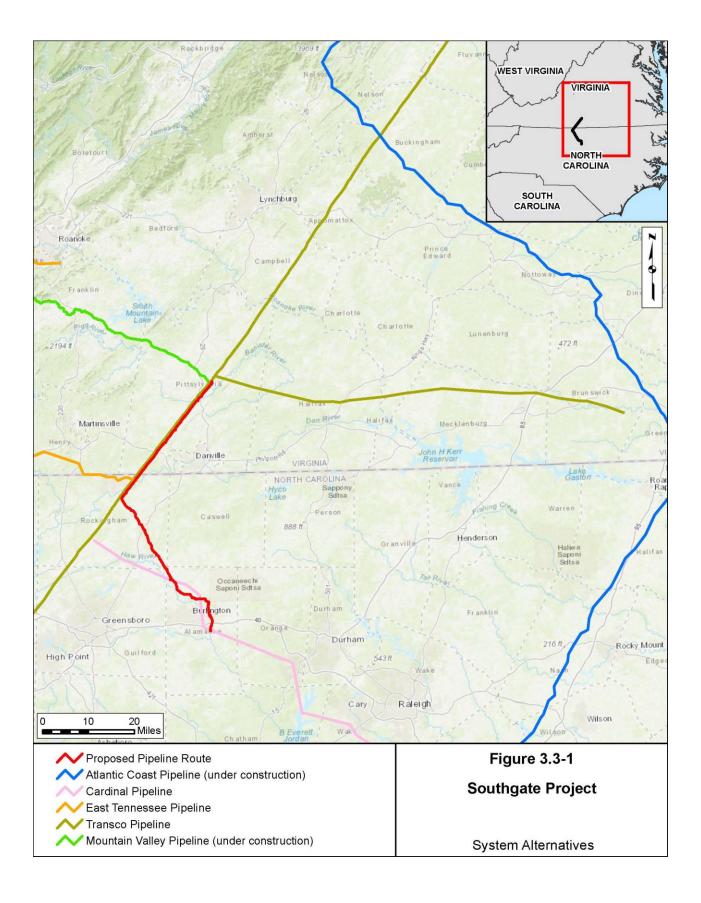
3.3.2 Modifications of Existing and Approved Natural Gas Pipeline Systems

Since none of the existing or approved pipeline systems in the Project area have the capacity to meet the Project's purpose, each system would require modifications to meet the purpose of the Project. The modifications could include additional pipeline construction to connect to the natural gas supply, delivery area, or both; pipeline construction to create additional transportation capacity; additional compression; or some combination of these options.

3.3.2.1 Transco Pipeline System Alternative

The existing Transco system consists of various diameter pipelines totaling approximately 10,200 miles between Texas and New York. The system has a peak design capacity of almost 15 Bcf/d of natural gas to markets in the Northeast, Mid-Atlantic, and Southeast regions of the United States. The Southgate Project would be located adjacent to the Transco system in Virginia and North Carolina from mileposts (MPs) 0.4 to 32.9.

3-3 Alternatives



Alternatives 3-4

In comments on the draft EIS, Transco noted that although the firm transportation capacity of its system is currently fully subscribed, it could modify its existing system to provide the capacity sought by DENC by collocating a new 37.7-mile pipeline lateral along the existing right-of-way for the Cardinal Pipeline, and modifying an existing compressor station in Rockingham County, North Carolina. Additional system upgrades would likely be necessary before Transco would be able to provide the additional 375,000 dekatherms per day (Dth/d) of firm transportation service on its mainline from the Project's proposed receipt point with the MVP mainline to the interconnection between Transco and Cardinal Pipeline.

Mountain Valley responded to these comments stating that Transco's System Alternative would not meet several of the Southgate Project objectives that DENC considered prior to contracting for capacity on the Southgate Project, including increased competition and resiliency, risk diversification, and a direct physical connection to East Tennessee's interstate pipeline system. DENC agreed with Mountain Valley, stating that the Transco System Alternative would not meet the Southgate Project need with less environmental impact and at a lower cost, noting two reasons: 1) Transco failed to explain how its proposal would resolve Transco's lack of available firm capacity on its mainline; and 2) that its alternative would be unable to meet their timing needs for bringing the Southgate Project's proposed capacity online.

We conclude that undefined modifications would be required along Transco's mainline. Transco did not explain what upgrades would be needed to resolve its mainline system's lack of available firm capacity. The impacts of these upgrades may be less than, similar to, or greater than those that would occur as proposed by the Southgate Project. Therefore, we are unable to determine that this alternative would provide a significant environmental advantage.

Finally, as Mountain Valley and DENC pointed out, beginning the numerous permitting processes anew would cause delays that would be inconsistent with DENC's timing needs for bringing into service this additional capacity. While this last factor was not included as a Southgate Project objective, it is clearly a consideration that could affect the economic feasibility of the Southgate Project. Therefore, this alternative is not considered further in this analysis.

3.3.2.2 East Tennessee System Alternative

The East Tennessee pipeline system has the capacity to transport 1.9 billion cubic feet per day (bcf/d) of natural gas and extends from Nashville, Tennessee, through Virginia, to Eden, North Carolina where it interconnects with the Transco pipeline system. The East Tennessee pipeline system does not connect with the Southgate Project's proposed receipt point with the Mountain Valley Pipeline. The Southgate Project would interconnect with the East Tennessee pipeline system at the LN 3600 Interconnect taking gas to delivery points. To meet the purpose of the Project, modifications to the East Tennessee pipeline system would be required to supply 375 MMcf/d of natural gas to the DENC distribution system. The modifications would include upgrades similar to the Project including approximately 30 miles of pipeline collocated with the Transco pipeline system, 40 miles of new pipeline, and additional compression. These modifications would result in environmental impacts similar to those that would occur as proposed by the Project. Therefore, we conclude that this alternative would not provide a significant environmental advantage.

3-5 Alternatives

3.3.2.3 Atlantic Coast Pipeline System Alternative

The ACP Project, currently under construction, consists of 604 miles of natural gas pipeline in West Virginia, Virginia, and North Carolina. As noted above, the ACP Project is approximately 100 miles east of the T-15 Dan River and T-21 Haw River interconnects. In comments on the draft EIS, ACP states that rather than connecting to the western side of DENC's system as proposed by Mountain Valley, deliveries from ACP to DENC could occur on the eastern side of DENC's service territory. ACP contends that the ACP System Alternative could provide the additional gas through a combination of 140,000 Dth/d of available capacity on its system, ancillary facility enhancements, and upgrades to the existing Piedmont system, on which ACP has leased capacity.

As ACP acknowledged, the ACP System Alternative would not connect to the Project's proposed receipt points with the mainline Mountain Valley Pipeline or with East Tennessee's interstate pipeline system. Nor would the ACP System Alternative facilitate deliveries to the Southgate Project's proposed delivery points on DENC's distribution system in Rockingham and Alamance Counties, North Carolina. For these reasons, we find that the ACP System Alternative does not meet the stated purpose of the Southgate Project.

In order to connect the ACP Project with DENC's receipt points, a minimum of 100 miles of new pipeline (and associated compression) infrastructure would be required. Therefore, we conclude that the use of the ACP System Alternative would not provide a significant environmental advantage. For these reasons, the ACP Project is not considered further in this analysis.

3.3.2.4 Cardinal Pipeline System

The Cardinal Pipeline Company, co-owned by affiliates of Transco, Piedmont Natural Gas Company, and Dominion Energy, operates 105 miles of 24-inch-diameter intrastate pipeline in North Carolina originating in Rockingham County at an interconnect with the Transco pipeline system, extending southwest to Wake County. The Cardinal Pipeline Company transports natural gas from the Transco pipeline system to the Dominion Energy distribution system and Piedmont Natural Gas system. To meet the objective of the Southgate Project, modifications to the existing Cardinal Pipeline similar to those described above (i.e., a lateral and compression) would be necessary. Providing the gas to this lateral would either require the use of the Transco system, as described above, or additional pipeline construction. The impacts of these upgrades may be less than, similar to, or greater than those that would occur as proposed by the Project. Therefore, we cannot conclude that this alternative would provide a significant environmental advantage.

3.4 ROUTE ALTERNATIVES AND VARIATIONS

Early in the development of the Project, Mountain Valley considered a pipeline route that was largely collocated with existing utility rights-of-way. Upon more detailed route evaluation and after the determination of the presence of constraints such as residential areas, ponds, and side slopes, Mountain Valley subsequently incorporated minor deviations in the Project route. During the course of the pre-filing and environmental scoping process, Mountain Valley incorporated at least 46 of the 122 route variations into the Southgate route to avoid and/or minimize impacts on specific resources at the request of landowners and stakeholders.

Alternatives 3-6

Major route alternatives represent substantial deviations from a proposed route that may offer significant environmental advantages compared to the proposed route. Smaller route alternatives represent deviations to the proposed route between certain mileposts in a particularly sensitive area that may offer a significant environmental advantage to the proposed route. Minor route variations include minor deviations (or reroutes) over a short distance that might avoid a specific resource at that location.

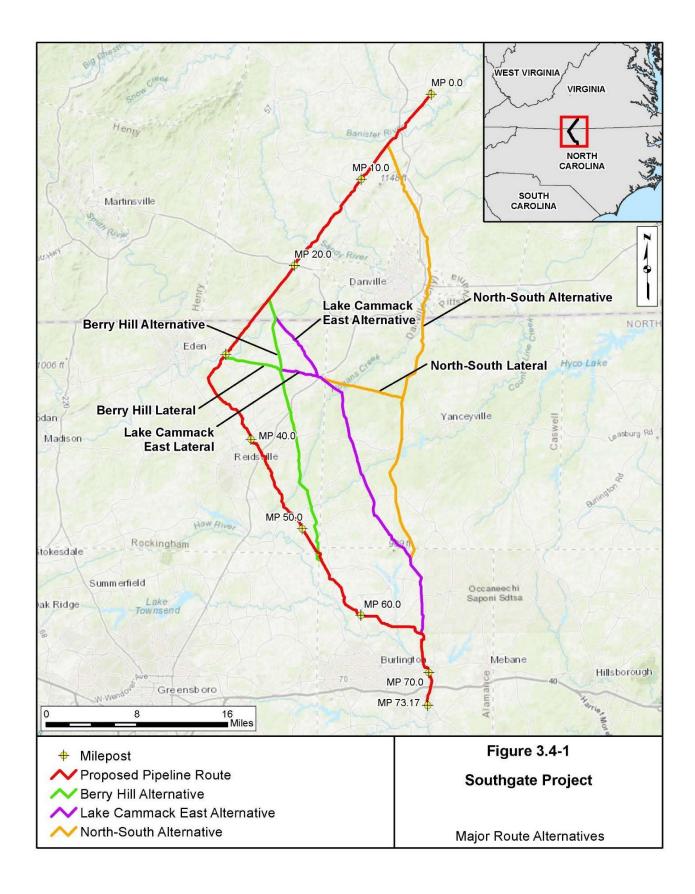
We evaluated three major route alternatives including the Berry Hill Alternative, Lake Cammack East Alternative, and the North-South Alternative. The locations of the major route alternatives are shown on figure 3.4-1. We also evaluated six minor route alternatives including the Haw River Alternative, Haw River West Alternative, Green Level Alternative, Jimmie Kerr Road Alternative, Duke Energy Powerline Extension Alternative, and City of Burlington Alternative. The locations of the minor route alternatives are shown on figures 3.4-2 through 3.4-7. Finally, we evaluated eight minor route variations including the Nicholson Variation, Whitehead Variation, Robert Pollok-Hill View Farms Variation, Moore Variation, Strader Variation, Madren Variation, Taylor East Variation, and Taylor West Variation. The locations of the minor route variations are shown on figures 3.4-8 through 3.4-14.

Mountain Valley incorporated several route variations that we evaluated in the draft EIS into its proposed pipeline route filed with the Commission on October 23, 2019. Therefore, these route variations are incorporated into the Proposed Action and are no longer evaluated in this section. These variations include the Bombardier Variation, Shambley Variation 1, Shambley Variation 2, Martin Marietta Variation, and Town of Haw River Variation.

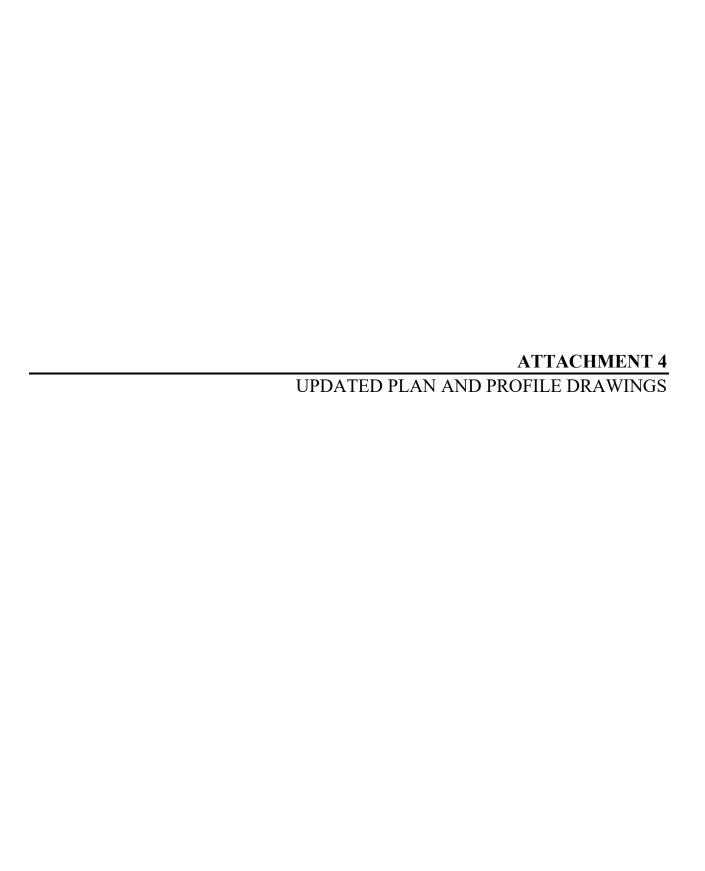
On October 23, 2019, in response to alternatives considered in the draft EIS, Mountain Valley submitted impact analysis comparison tables for each alternative based on changes in the current proposed route and new information gathered on each alternative or route variation. The revised data represents refinements to the previous data that are derived largely from new 2016 U.S. Geological Survey (USGS) National Land Cover Dataset, revised 2019 pipeline and electrical utility data, and other updated sources. The revisions do not alter our conclusions.

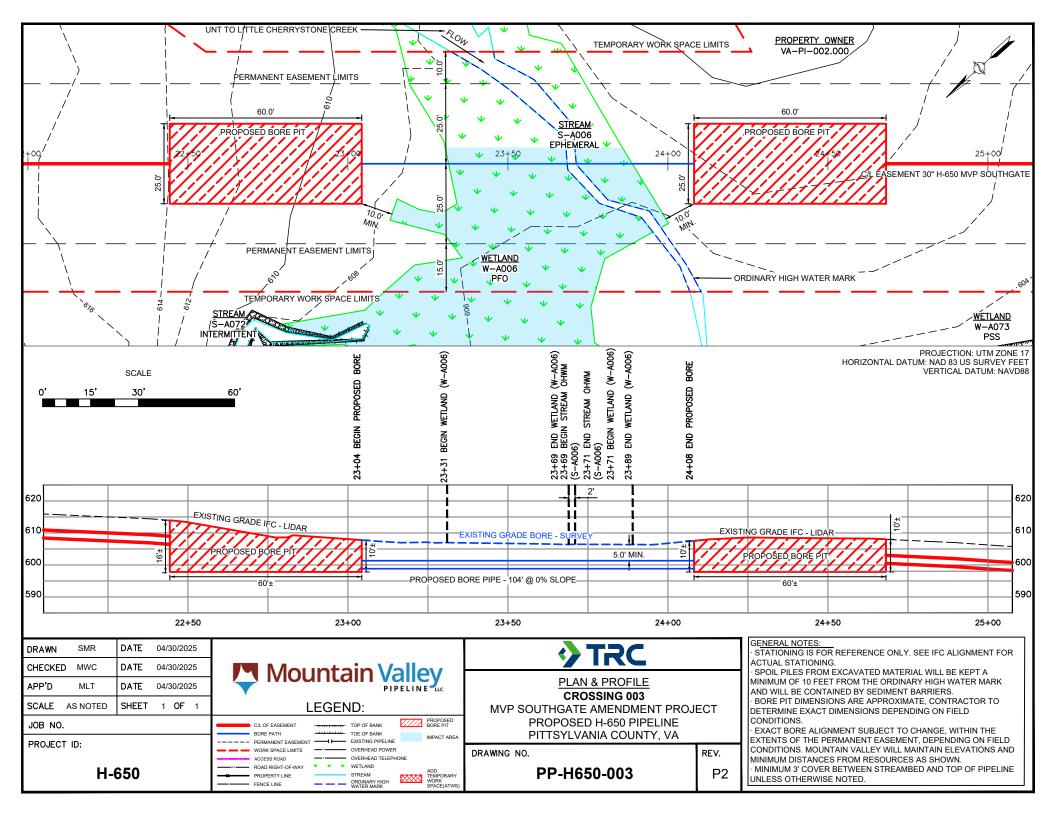
3-7 Alternatives

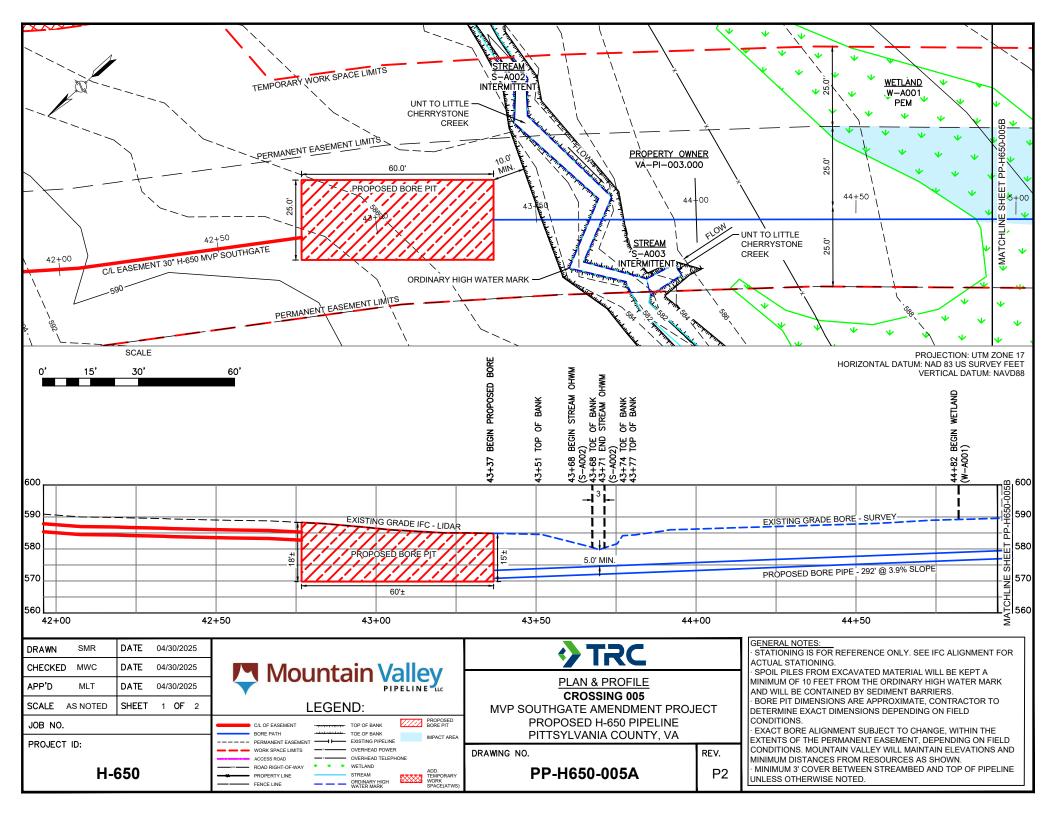
¹ This information can be viewed on the FERC website at http://www.ferc.gov. Using the "eLibrary" link, select "Advanced Search" from the eLibrary menu and enter the accession number in the "Numbers: Accession Number" field. Accession number 20191023-5022 contains supplemental project information filed on October 23, 2019. Accession number 20191220-5298 contains revised alignment sheets for the Project.

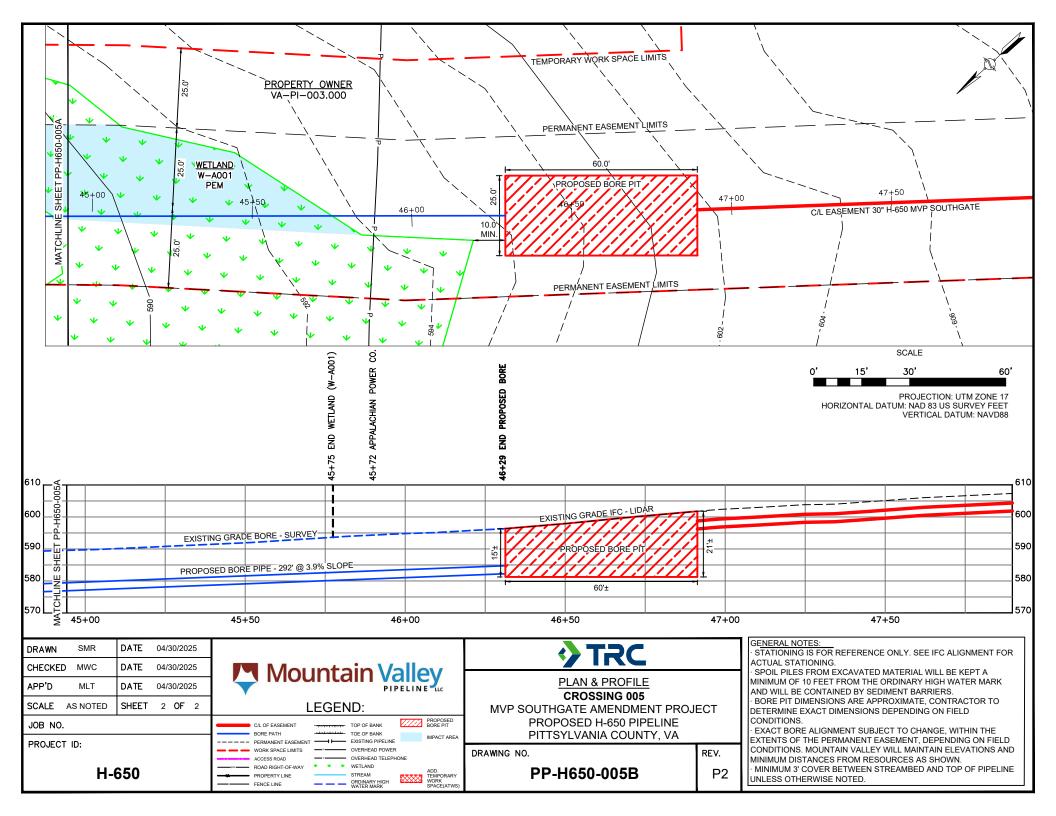


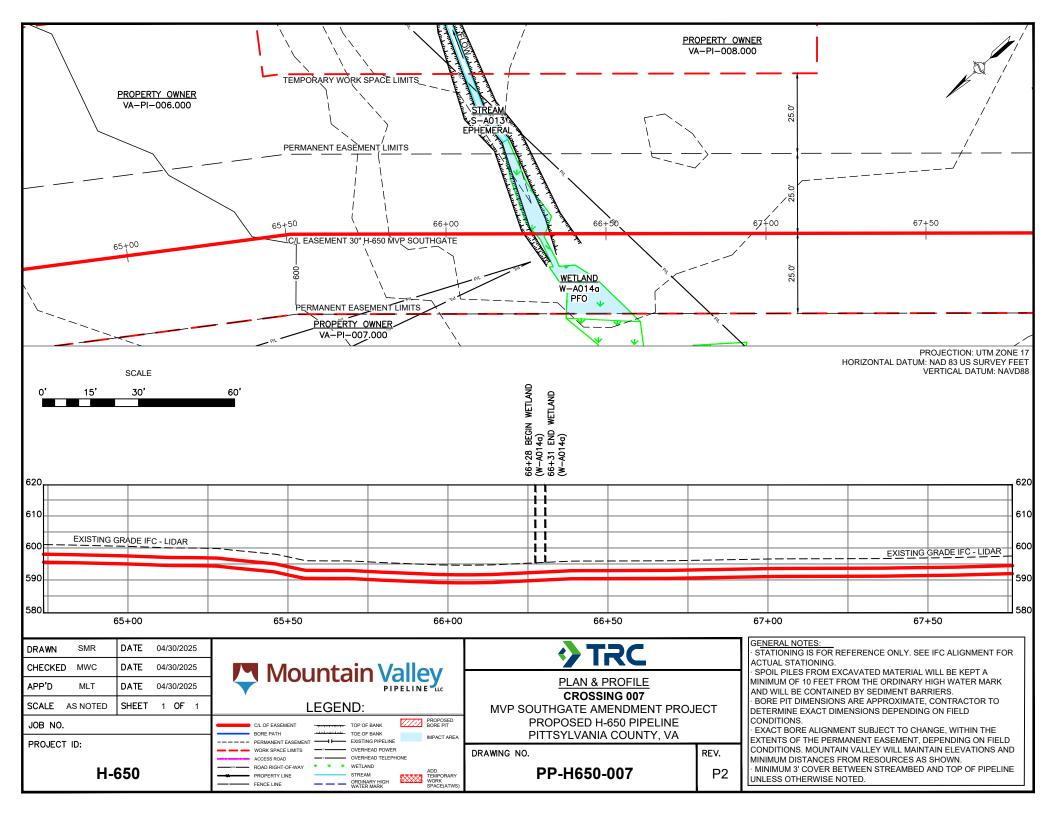
Alternatives 3-8

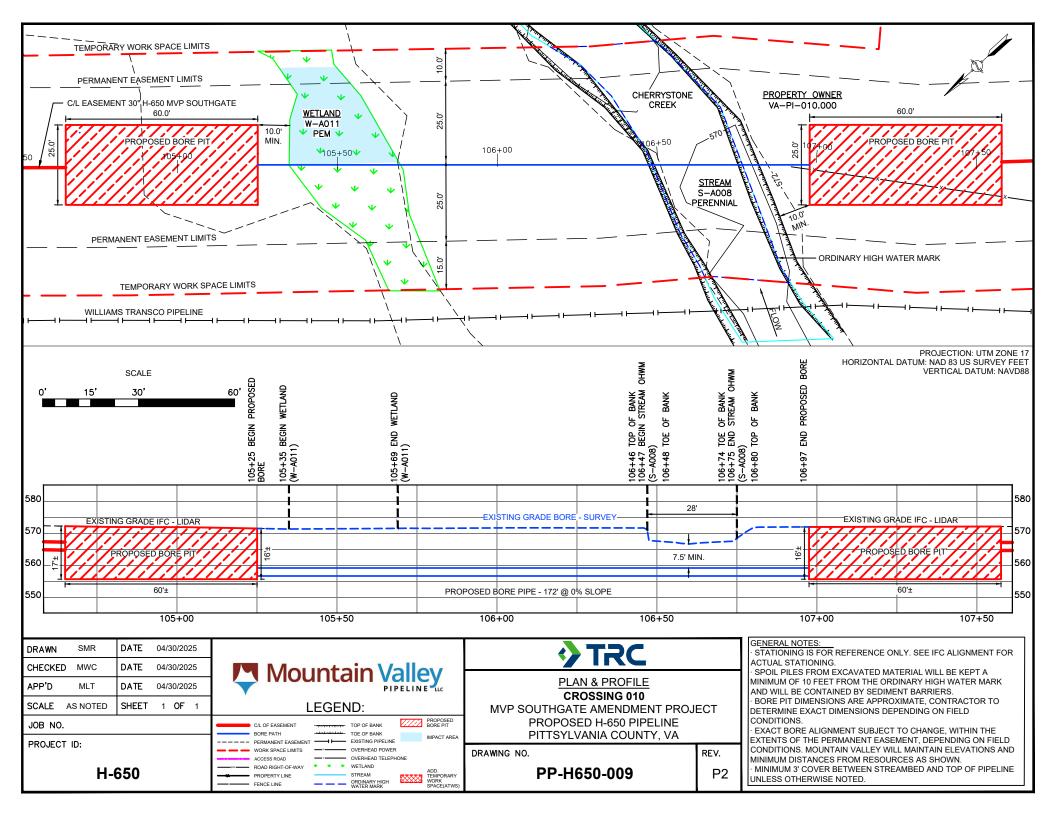


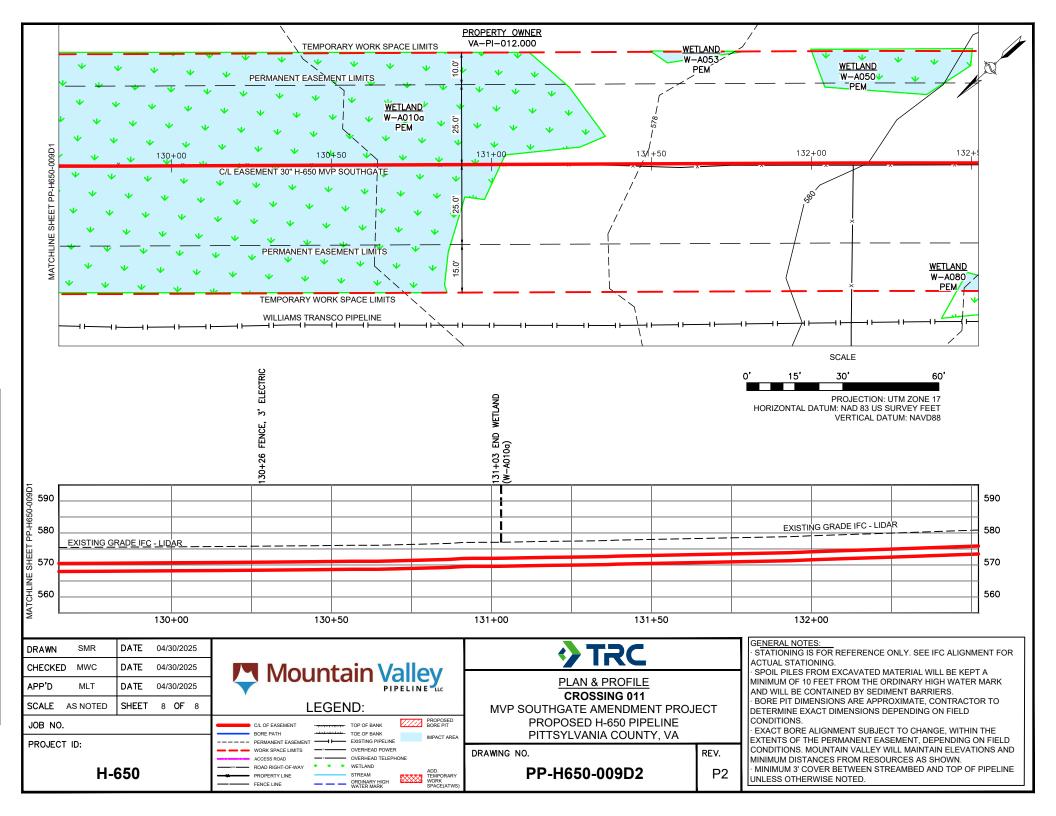


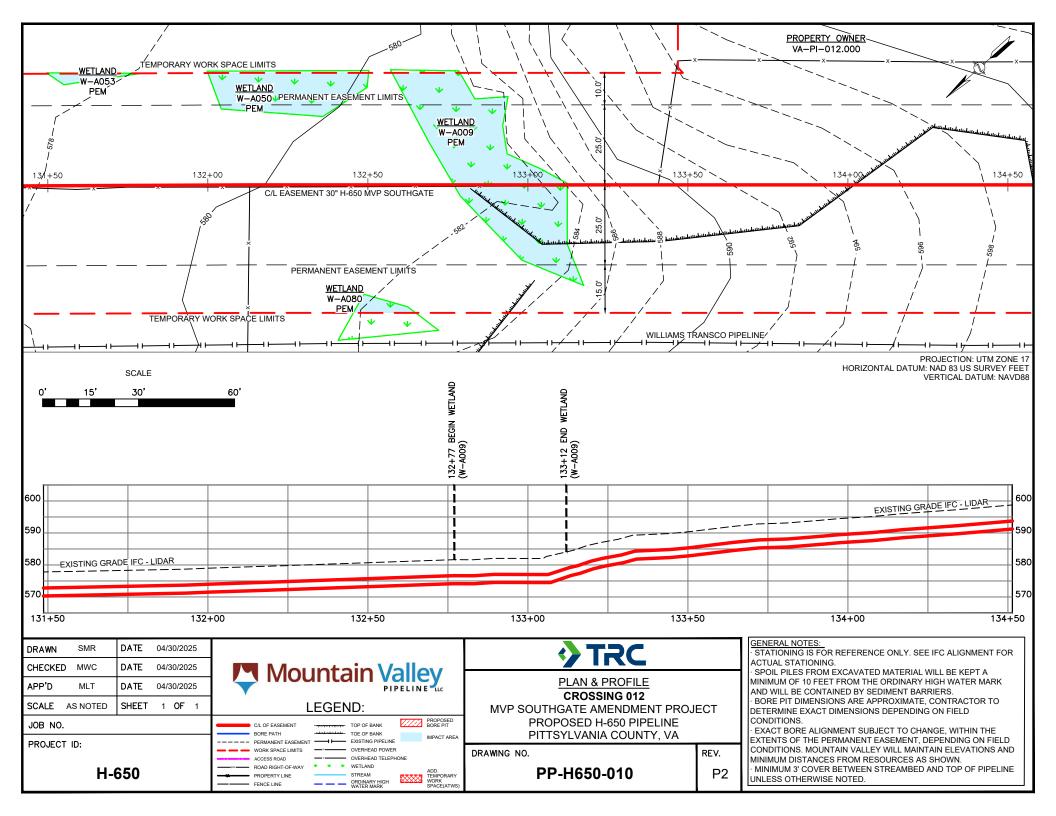


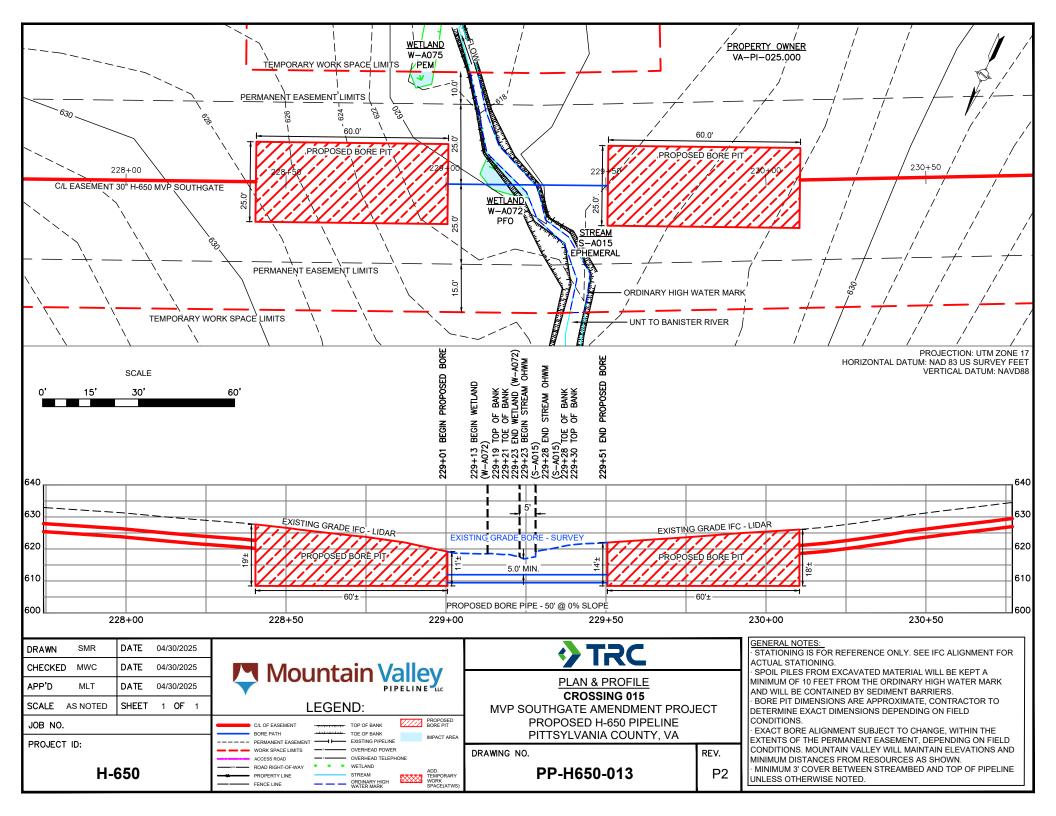


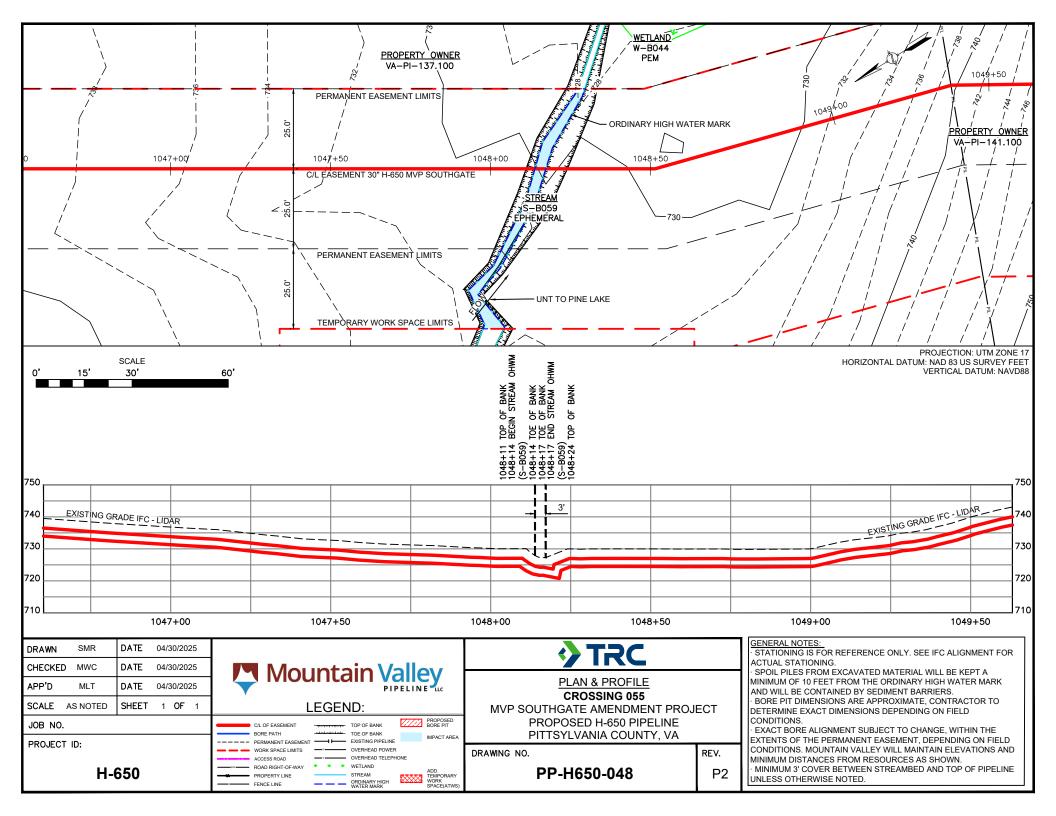


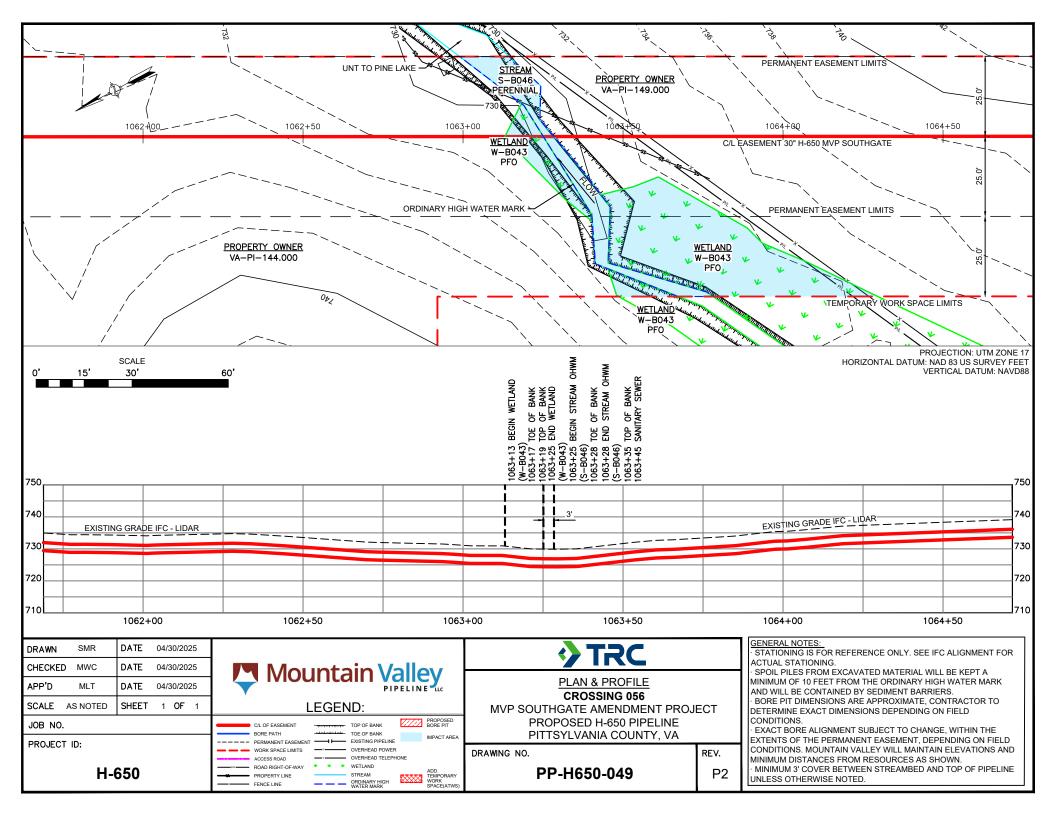


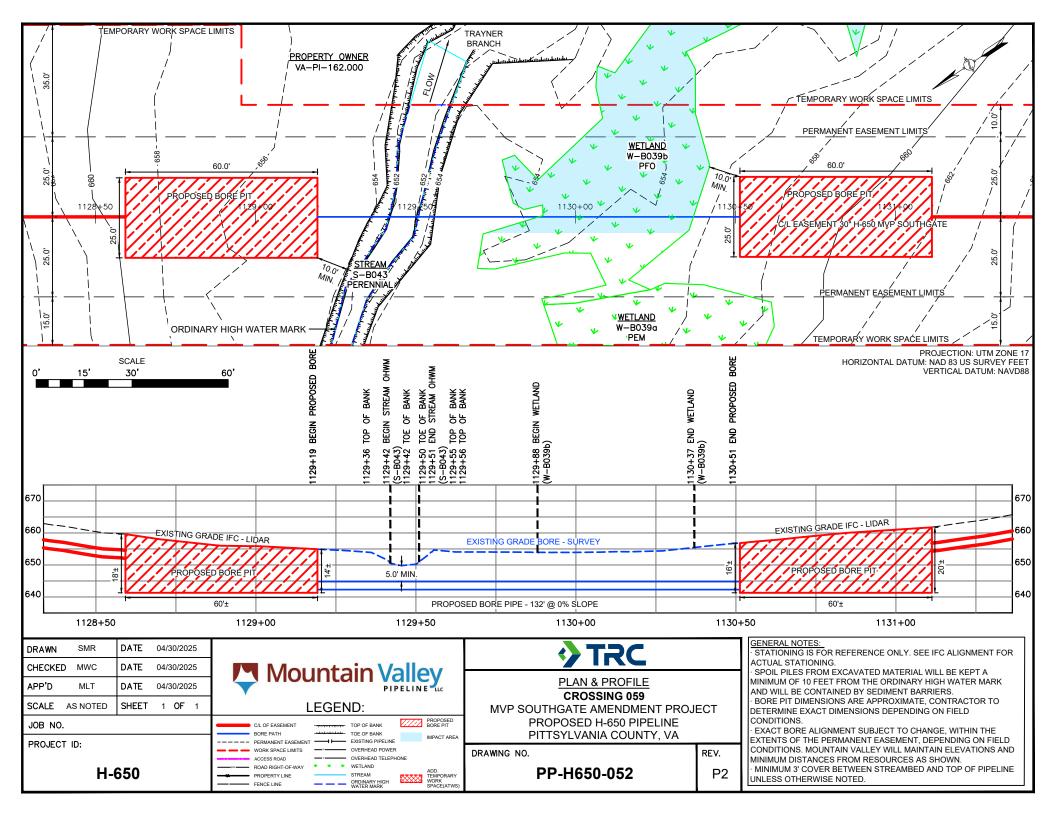


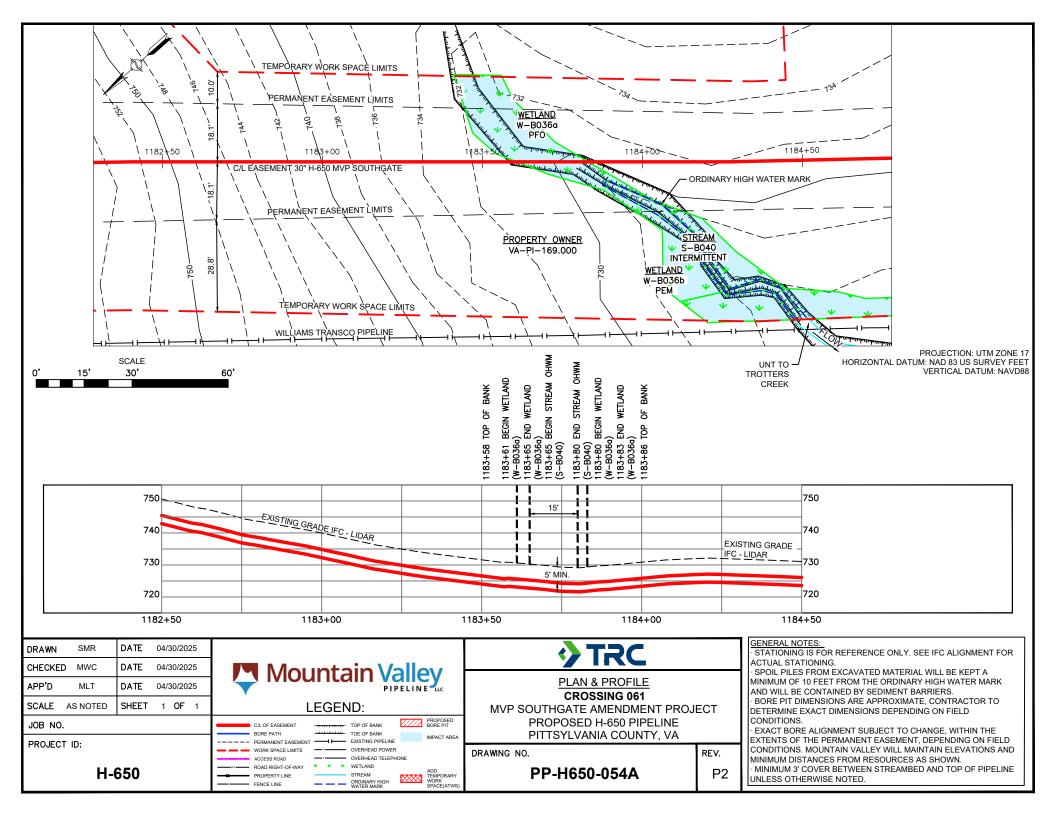


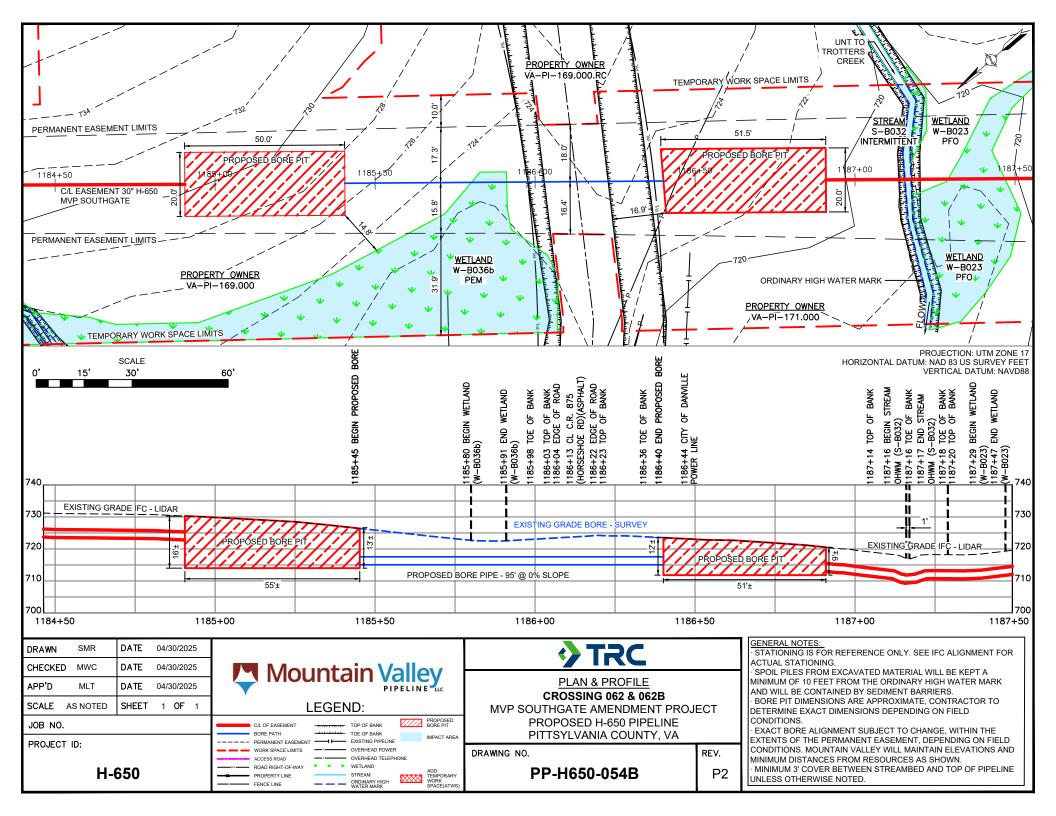


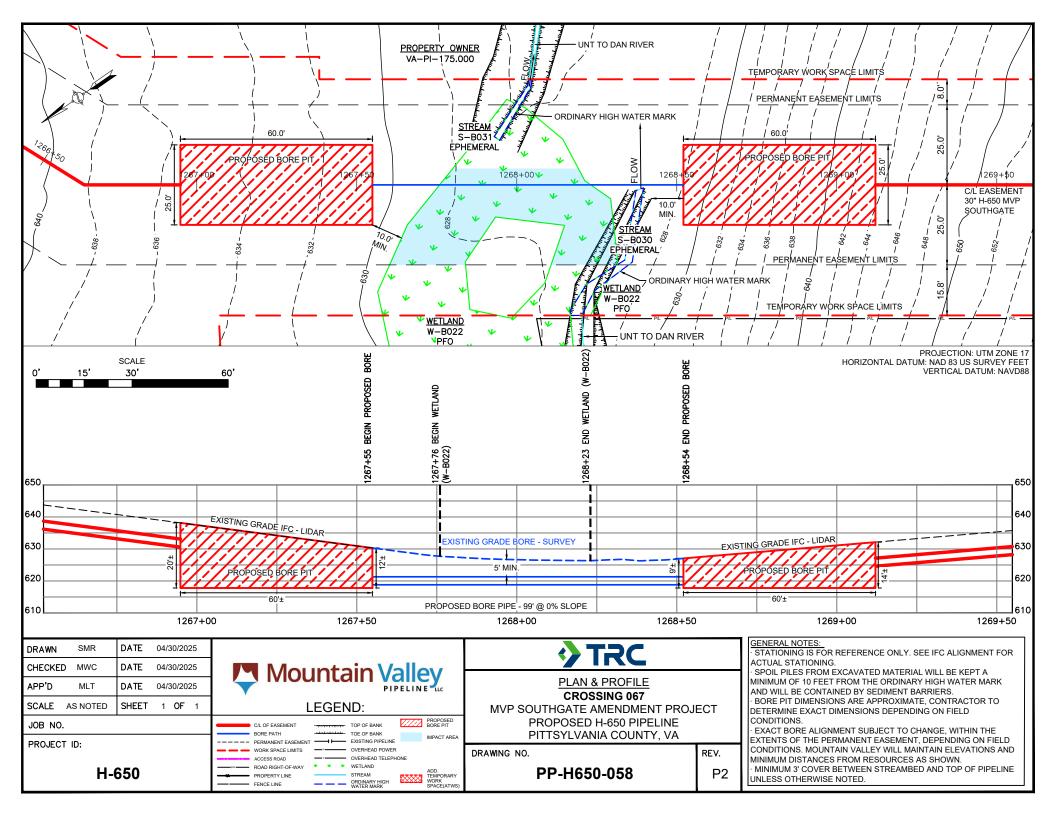


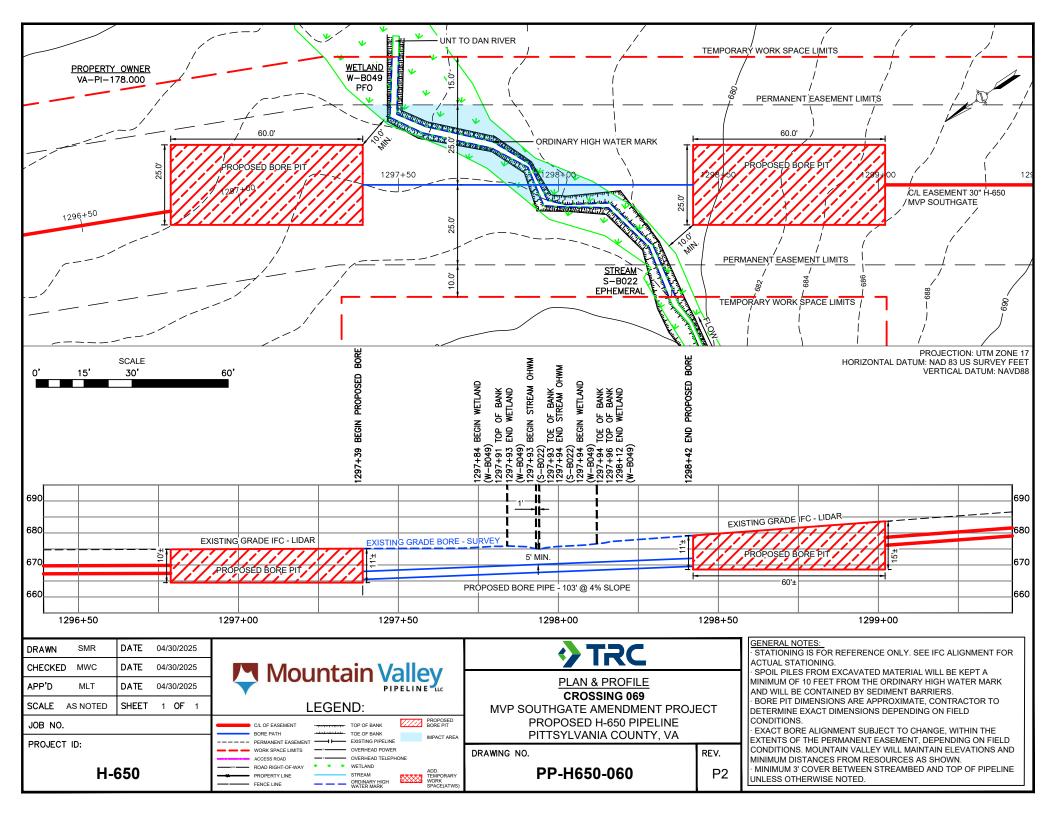


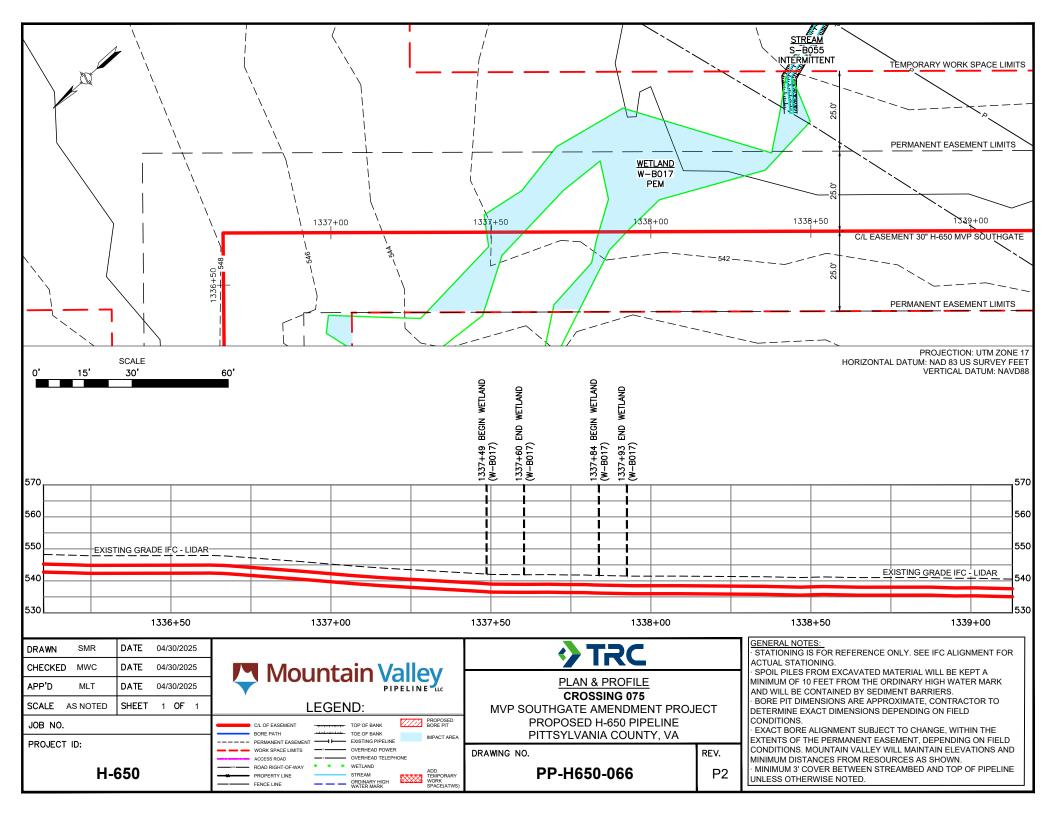


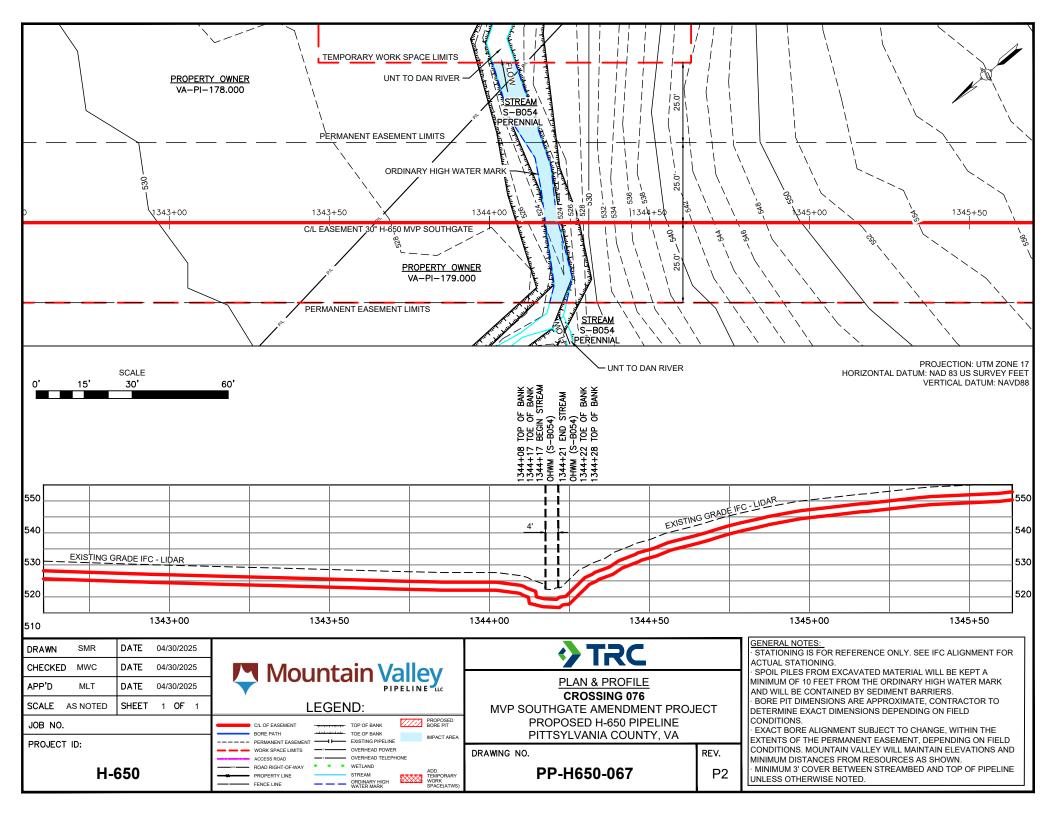


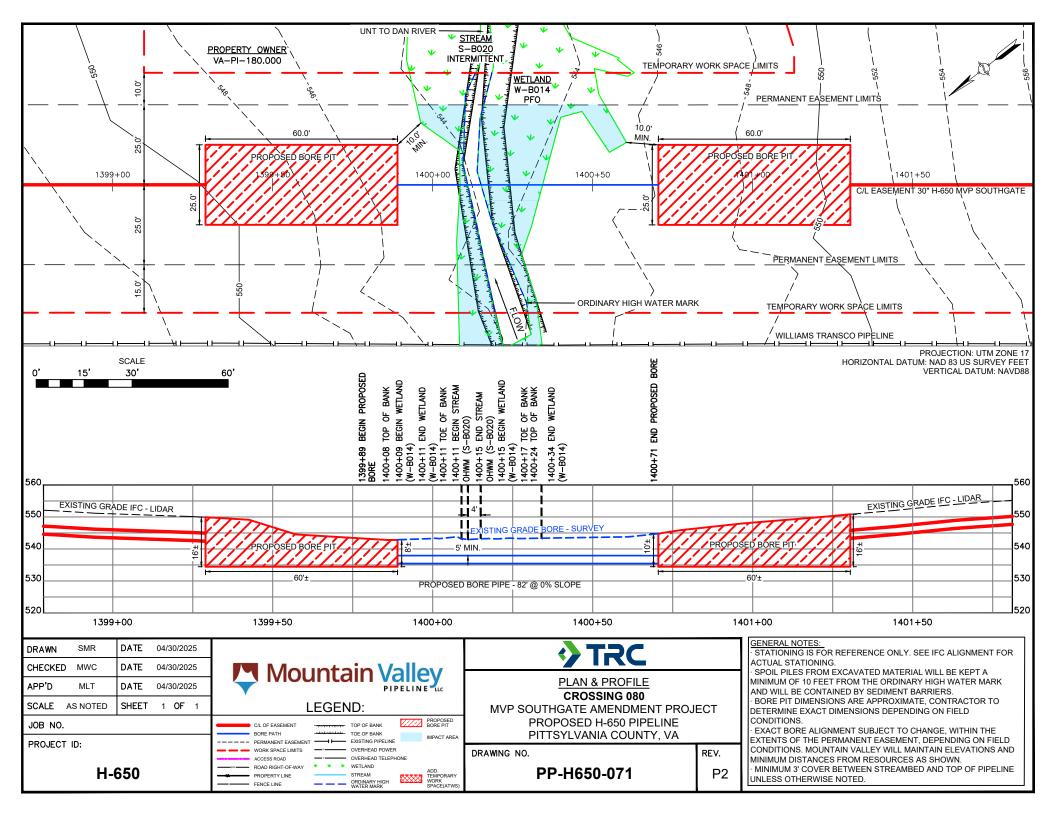


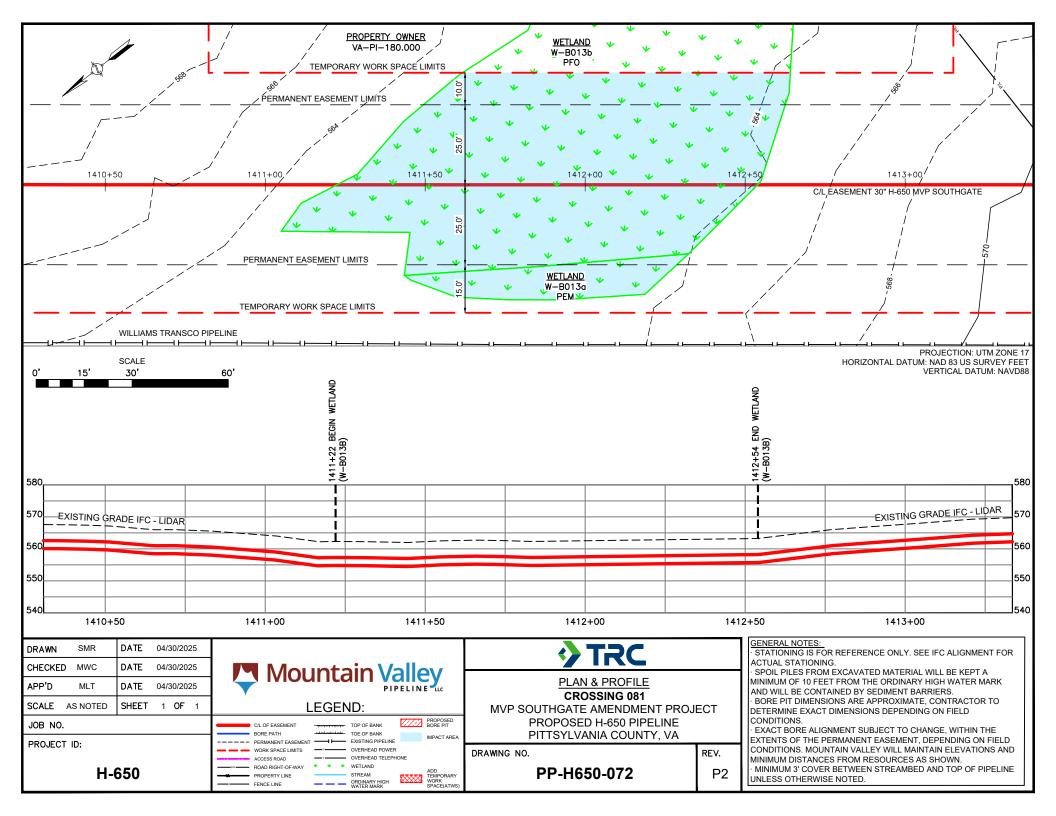


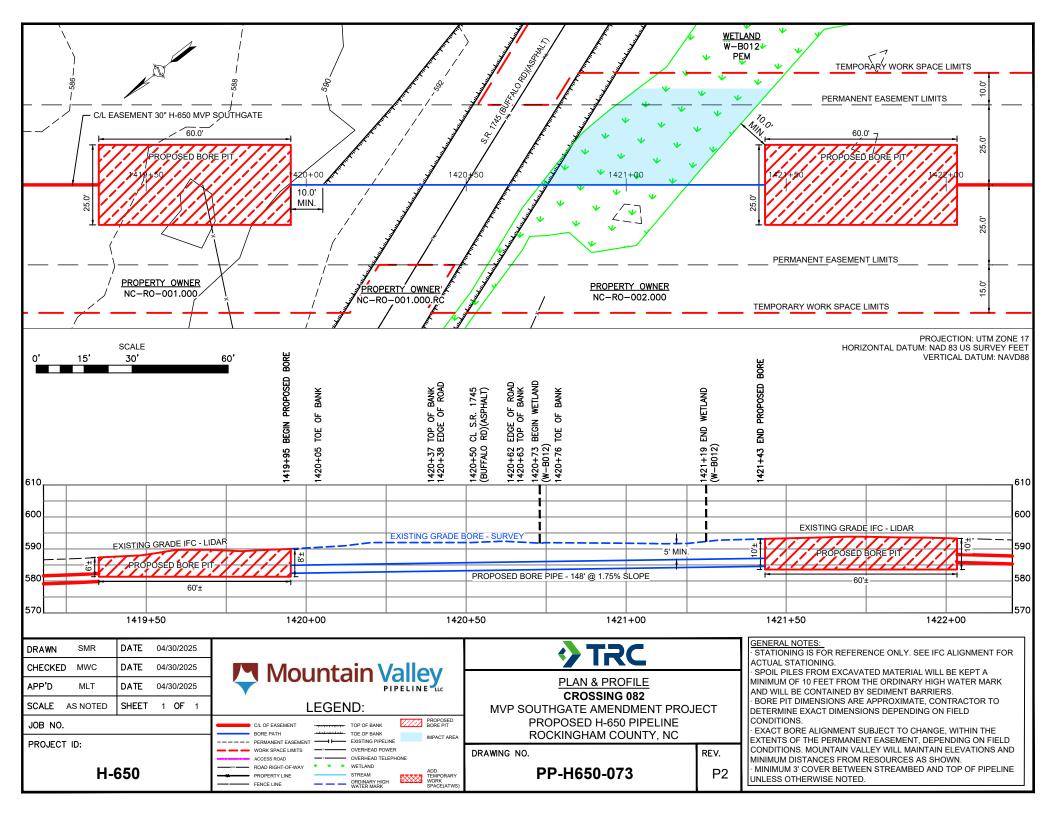


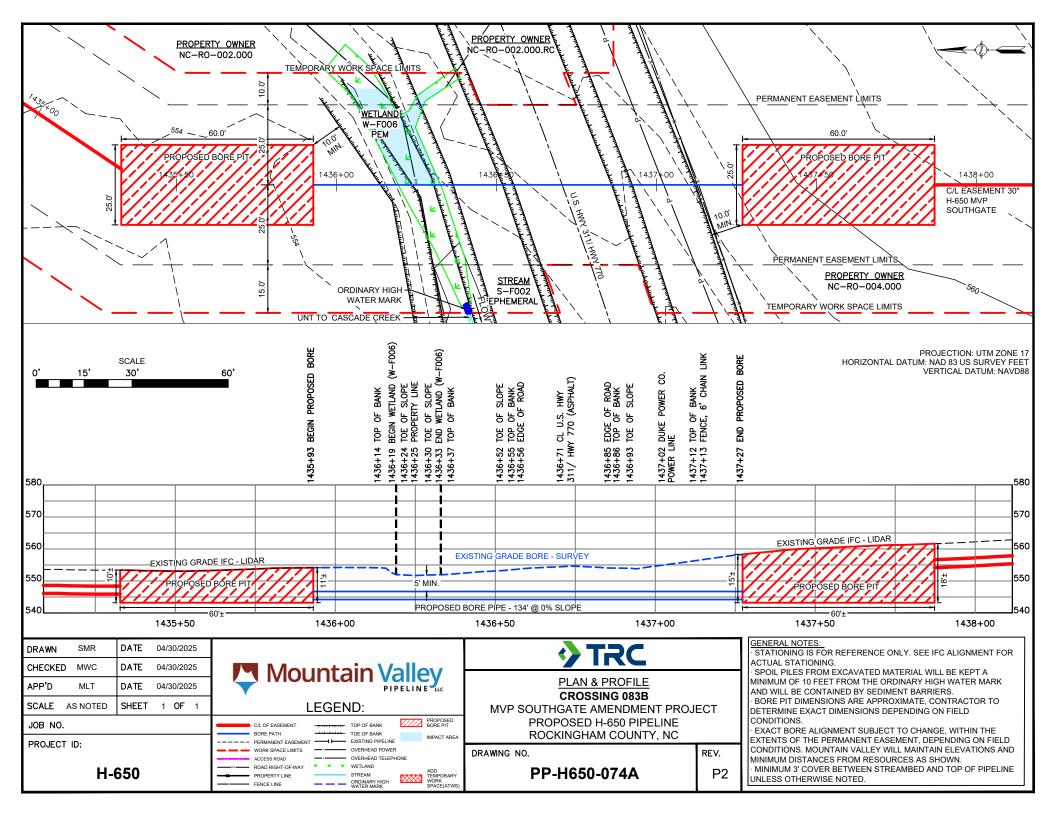


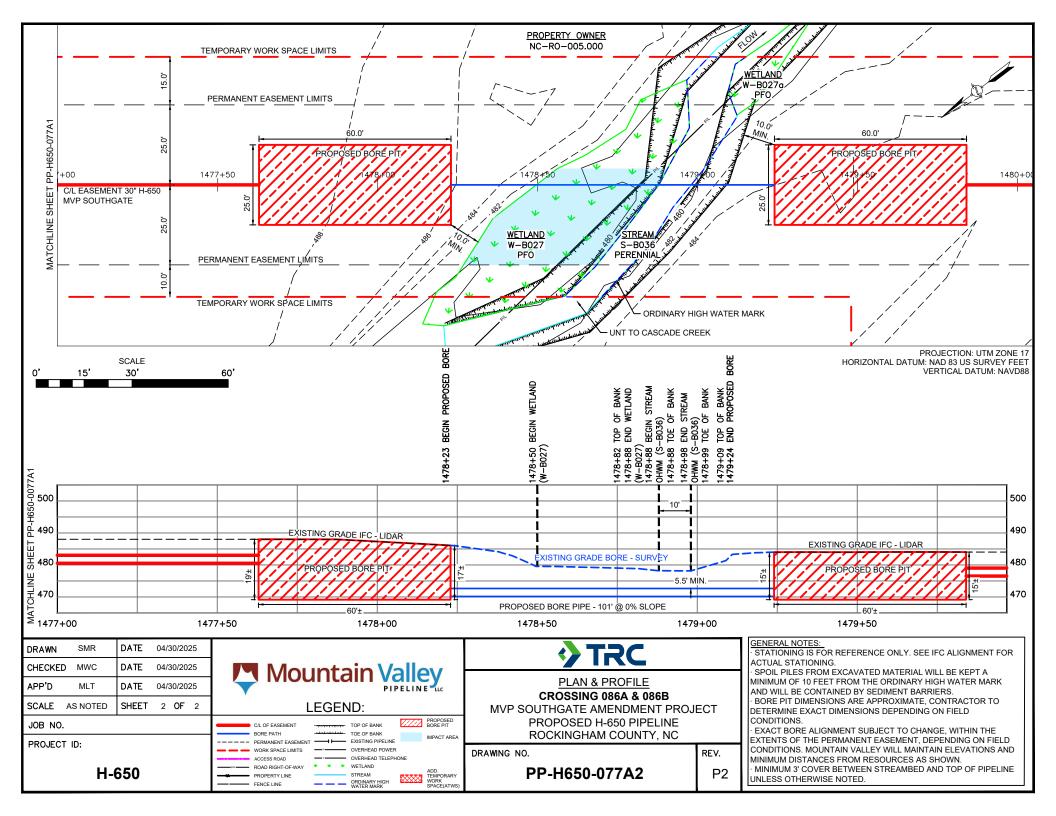


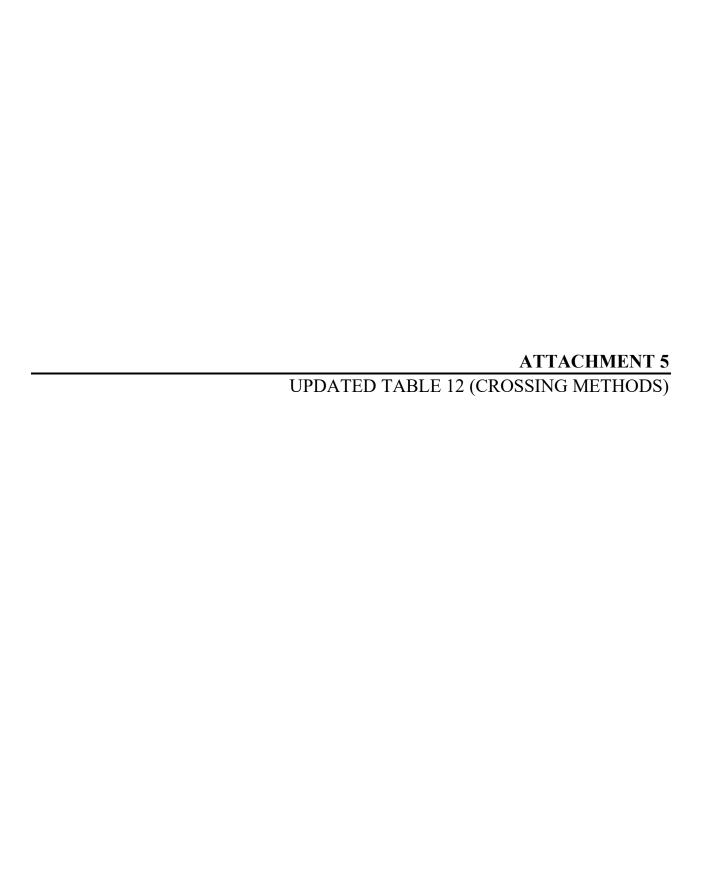












USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale
Norfolk	H650-001	S-A005	Dry-Ditch Open-Cut Conventional Bore	103	26	N	10.0	Y	\$79,080.00 \$479,230.00	Dry-Ditch Open-Cut	This small UNT to Cherrystone Creek (less than five feet wide) would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required which would creat a large volume or animaria to be executed and stockpiled. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably fight feative to the proposed construction method.
Norfolk	H650-002	W-A004	Dry-Ditch Open-Cut Conventional Bore	- 78	- 13	N	3.8	N .	\$28,080.00 \$228,055.00	Dry-Ditch Open-Cut	The open cut method would result in a temporary impact of approximately 0.04 acre of a PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is an unreasonably high cost relative to the proposed construction method. The lack of stificient space to stockpile the material further complicates a trenchless crossing. Furthermore, the conventional bore crossing cost to avoid the temporary impact is sureasonably high relative to the proposed construction method.
Norfolk	H650-003	W-A006, S-A006	Dry-Ditch Open-Cut Conventional Bore	104	16	N	9.2	Y	\$79,440.00 \$270,582.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
Norfolk	H650-004	W-4003a, W-4003b, S-4004	Dry-Ditch Open-Cut	- 402	-	- N	10.3	N	\$186,720.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact to one wetland and Little Cherrystone Creek. Avoiding/minimizing this minor impact through a conventional bore would require a relatively deep bore pit more than 20 Feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the footprint of the bore pit in an already restricted workspace. In addition, the length of the crossing exceeds capabilities of conventional bore machines, and the configuration of the LOD does not allow for non-conventional techniques without impacting
			Conventional Bore		23				\$865,530.00		other wetlands and streams. Therefore the conventional bore method is not feasible. A conventional bore crossing would extend the duration of this crossing from 10 to 29 days, thereby increasing the noise, aesthetic, and other impacts on nearby persons.
			Dry-Ditch Open-Cut		-				\$144,240.00		There are no significant constraints on available crossing methods or significant
Norfolk	H650-005	S-A002, S-A003, W-A001	Conventional Bore	284	20	N	8.9	Y	\$592,753.00	Conventional Bore	environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
Norfolk	H650-007	W-A014	Dry-Ditch Open-Cut Conventional Bore	82	12	- N	5.6	Υ	\$71,520.00 \$220,194.00	- Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.07 acre of a PEM wetland. Additionally, a trenchless crossing of this resource would result in the safety risk of operating heavy equipment for an extended time with a private landowner in close proximity, thereby increasing the noise, aesthetic, and other impacts on nearby persons.
			Dry-Ditch Open-Cut						\$64,680.00		The open-cut method would result in a temporary impact to one small (less than five feet in width) UNT to Cherrystone Creek. A conventional bore crossing would result in an increase
Norfolk	H650-008	S-A012	Conventional Bore	- 63	18	N	6.5	Y	\$54,680.00	Dry-Ditch Open-Cut	in cost by 3.9 times and an increase in the duration of the crossing by 4 days. Additionally, due to the soit bype and proximity to other wetlands and a pond, there would likely be groundwater infiltration during bore pit dewatering operations given the soil bype and proximity to other wetlands and the pond on this parcel. A pump discharge would have to placed far enough away from the immediate crossing vicinity so as not to impact the lands and the pond on the processing vicinity so as not to impact the lands of the processing vicinity so as not to impact the lands.
Norfolk	H650-008A	S-A009. W-A013a, W-A013b, W-A013c, W-A013d	Dry-Ditch Open-Cut	1150	-	N	0.0		\$456,000.00	Drv-Ditch Open-Cut	The length of the crossing exceeds capabilities of conventional bore machines, and the configuration of the LOD does not allow for one-conventional techniques without impacting other wetlands and streams. Therefore the conventional bore method is not feasible. A longer bore that would incorporate next wetland complex would not be feasible because
NOTTOIK	H650-008A	S-AUU9, W-AU138, W-AU130, W-AU13C, W-AU130	Conventional Bore	1150	25	N	0.0	Y	\$1,953,043.00	,	the elevation of the terrain on the downstream end of the crossing is to great to fabricate a pullback string. Additionally, this crossing would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled.
Norfolk	H650-010	W-A011. S-A008	Dry-Ditch Open-Cut	172	-	,	1.4	N	\$103,920.00	Conventional Bore	The depth of Cherrystone Creek is greater than 2 feet, indicating potential high flows, especially during storm events. Setting up dam and pump operations for open-cut crossings
NOTION	H650-010	VV-MUII, 3-MUU6	Conventional Bore	1/2	17	,	1.4	N	\$401,831.00		during large flow events wil require larger bladders and more pumps, further restricting the workspace to an impracticable width. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
Norfolk	H650-011	W-A010b, W-A010a, W-A010c	Dry-Ditch Open-Cut	2000	-	N	0.0	Y	\$720,000.00	Dry-Ditch Open-Cut	The length of the crossing exceeds capabilities of conventional bore machines, and the configuration of the LOD does not allow for non-conventional techniques without impacting
			Conventional Bore		20				\$3,093,546.00		other wetlands and streams. Therefore, the conventional bore method is not feasible.
Norfolk	H650-012	W-A009	Dry-Ditch Open-Cut	- 64	-	N	10.5	N	\$23,040.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of 0.0.4 are of PEN wetland. Avoiding/minimizing this minor impact through a conventional bore would require a bore pix at least 20 feet deep. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per CSHA requirements, this exervation may require sloping and increase the footprint of the bore pix for the properties of the contract of the properties of the properti
			Conventional Bore		21				\$326,378.00	00	in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Norfolk	H650-013	S-A018	Dry-Ditch Open-Cut	52	-	N	12.0	Y	\$60,720.00		This small UNT to Banister River (less than five feet wide) would require a bore pit that is approximately 24 feet deep. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the footprint of the bore pit

USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale	
Horox	11000 020	5 W.L	Conventional Bore	J.	24		12.0	·	\$505,265.00	Bij dikili opeli od	in an already restricted workspace. Additionally, a conventional bore crossing would extend the duration of this crossing lyb of sub, thereby Increasing the noise, aesthetic, and other impacts on nearby persons. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-014	W-A019, S-A019	Dry-Ditch Open-Cut	82	ē	N N	10.6	Y	\$71,520.00	- Dry-Ditch Open-Cut	Avoiding/minimizing the temporary impact to 0.07 acre of PFO wetland through a conventional bore would require a deep bore pit of approximately 20 feet. This depth would require engineered shorting and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increases the foothprint of the bore pit in an already restricted workspace.	
			Conventional Bore		20				\$279,565.00		require sopring and increase the tootprint or the bore pit in an already restricted winrspace. Additionally, a threnchless crossing of this resource would result in the safety risk of operating heavy equipment for an extended time with a private landowner in close proximity, thereby increasing the noise, aesthetic, and other impacts on nearby persons.	
Norfolk	H650-015	W-A072, S-A015	Dry-Ditch Open-Cut Conventional Bore	- 50	19	N	12.4	Υ	\$60,000.00 \$241,979.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
Norfolk	H650-016	S-A017	Dry-Ditch Open-Cut	45	-	N	11.8		\$58,200.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Banister River (less than five feet wide) through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the	
NOTOR	H630-U16	S-A017	Conventional Bore	45	23	N	11.0	, ,	\$467,640.00	Біў-Бікіі Ореп-Сак	excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloging and increase the footpoint of the bore pit in an aireary estricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is urreasonably high relative to the proposed construction method.	
Norfolk	H650-017A	S-8060, W-A073a,W-A073b	Dry-Ditch Open-Cut	120	1	N	6.3	Y	\$85,200.00	Dry-Ditch Open-Cut	The Bannister River directly south of this crossing will be bored to avoid impacts to federally listed species and to avoid the anticipated high flows in the stream that would make an open-cut crossing difficult. In order to perform a trenchless crossing of the Banister River,	
Notice	11030 0177	0 5000, 11 10700, 11 70700	Conventional Bore	110	15		0.0	·	\$175,200.00	Diy Dictil Open Out	the stream and wetland complex at this crossing must be open-cut in order to facilitate construction of the bore pits associated with the Banister River. A trenchless crossing of the entire complex is not feasible due to bore pits that would be excessively deep.	
Norfolk	H650-017B	S-A020	Dry-Ditch Open-Cut	- 88	-	Y	3.3	N -	\$73,680.00	Conventional Bore	The depth of the Banister River is greater than 2 feet, indicating potential high flows, especially during storm events. Setting up dam-and-pump operations for open-cut crossings during large flow events would require larger bladders and more pumps, further restricting the workspace to an impracticable width. The direct aquatic impact will be	
			Conventional Bore		19				\$292,892.00		avoided/minimized by use of the conventional bore method.	
Norfolk	H650-018	W-A020, S-A021, W-A021	Dry-Ditch Open-Cut	204	-	N	8.9	N	\$115,440.00	Dry-Ditch Open-Cut	Avoiding/minimizing the temporary impact to this UNT to the Banister River and 0.1 acre of PFO wettand through a conventional bore would require a deep bore pir of over 20 feet. This depth would require engineered shoring and sologing falans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require slongly and increase the footprint and spois material from the bore pit in an increase.	
North	11000 010		Conventional Bore		21		3.3	"	\$571,881.00		Integrite subging and unlease terrorby in a disapposite statement into the too they and already restricted workspace. Does length is also longer than the desired length, increasing the chance of bore deflection of failure. Additionally, a conventional bore crossing would extend the duration of this crossing from 10 to 20 and, thereby increasing the noise, aesthetic, and other impacts on nearby persons.	
Norfolk	H650-019	W-A023, S-A022, W-A022	Dry-Ditch Open-Cut	880	-	Y	11.7	N	\$358,800.00	Dry-Ditch Open-Cut	The wetland complex is too long to bore in conjunction with the abutting railroad. The railroad must be bored, and the only feasible method of crossing the railroad is to open cut the wetland complex and have railroad bore pit within the wetland on the upstream side. Avoiding/minimizing this minor impact through a conventional bore would require a	
			Conventional Bore		50				\$2,454,030.00		relatively deep bore pit more than 50 feet, thereby requiring the excavation of an interim ramp and bench and dramatically increasing the space occupied by the bore pit and spoil pile. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
			Dry-Ditch Open-Cut		-				\$18,360.00		The open-cut method would result in a temporary impact of 0.02 acre of PEM wetland. Avoiding/minimizing this minor impact through a conventional bore would require a bore pit	
Norfolk	H650-020	W-A068	Conventional Bore	51	29	N	14.3	Y	\$531,210.00	Dry-Ditch Open-Cut	nearly 30 feet deep - requiring the operator to work from a shallow bench within the pit. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
			Dry-Ditch Open-Cut		-				\$63,720.00		The open-cut method would result in a temporary impact of 0.04 acre of PEM wetland and 0.1 acre of PFO wetland. Avoiding/minimizing this minor impact through a conventional bore would require above pit of approximately 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the execution. To maintain a refer necessaries on PEM short contracts of the period	
Norfolk	H650-021	W-A070b, W-A070c, W-A070a, W-A032	Conventional Bore	177	20	N	9.6	N	\$404,564.00	- Dry-Ditch Open-Cut	safe excavation per CSHA requirements, this excavation may require stoping and increase the loopprint of the bore pit in an already restricted workspace. Additionally, a conventional bore crossing would extend the duration of this crossing from 3 to 17 days, thereby increasing the noise, easthetic, and other impacts on nearby persons. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-022	S-A028	Dry-Ditch Open-Cut	38	-	N	14.8	N	\$55,680.00	Dry-Ditch Open-Cut	Avoiding/minimizing this minor impact to an ephemeral UNT to White Oak Creek through a conventional bore would require a bore pit at least 20 feet deep. This depth would require engineed shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe exervation per CSHA requirements, this exervation may require sloping	
NOTOR	110507-022	S-MUZO	Conventional Bore	30	21	N.	14.0	.4	\$306,686.00	, ,	Out maintain a safe excivation per ChSHA requirements, this excivation may require slopi and increase the fotoprist of the bore pit in an already restricted workspace. The lack stiffcient space to stockpile the material would further complicate a trenchless cross Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	

USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale	
Norfolk	H650-023	S-A027, S-A026	Dry-Ditch Open-Cut	164	-	N	29.8	Y	\$101,040.00	Dry-Ditch Open-Cut	These UNT to White Oak Creek would require a bore pit that is at least 35 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. Additionally, a conventional bore crossing would extend the duration of this crossing from 22 to 6 days, thereby increasing the potential for an erosion or sedimentation event to cocur.	
			Conventional Bore		37				\$993,032.00		Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-024	S-A025	Dry-Ditch Open-Cut	95	-	N	24.0	N	\$76,200.00	Dry-Ditch Open-Cut	This small, ephemeral UNT to White Oak Creek (less than five feet wide) would require a bore pit that is at least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. The lack of sufficient space to stockpile the material would	
			Conventional Bore		30				\$924,280.00		further complicate a trenchless crossing. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-025	W-A025. S-A024. S-A023	Dry-Ditch Open-Cut	98	÷	N	49.8	v	\$77,280.00	Dry-Ditch Open-Cut	This crossing is situated on a long and steep slope that would involve logistically difficult construction conditions and would require an excessively deep bore pit for a trenchless crossing. Bore pits at least 25 feet deep would be required, which would require the use of a	
TOTOR	1100 020	11 1020, 0 1024, 0 1020	Conventional Bore	3.5	27		45.5	·	\$627,235.00	. By blenegen out	bench and interim access ramp and create a large volume of material to be excavated and stockpiled. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
	H650-026	W-A017	Dry-Ditch Open-Cut	169	-		8.8		\$60,840.00		The open-cut method would result in a temporary impact of 0.18 acre to one PEM wetland. Avoiding/minimizing this minor impact through a conventional bore would require a bore pit at least 20 feet deep. This depth would require engineered shoring and sloping ghans, which increases the complexity of the execution. To minimize a safe execution por CHSHA	
Norfolk	H650-026	W-A017	Conventional Bore	169	20	N	8.8	Y	\$388,317.00	. Dry-Ditch Open-Cut	requirements, this exeavation may require sloping and increase the footprint of the bore pit in an already restricted workspace. A conventional bore crossing would extend the duration of this crossing from 3 to 16 days, thereby increasing the potential for an erosion or sedimentation event to occur. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-027	W-A029h W-A029a S-A001 S-A029 W-A028	Dry-Ditch Open-Cut	290	÷	N	53.2		\$146,400.00	Day Dilak Ones Cut	This crossing is situated on a long and steep slope that would involve logistically difficult construction conditions and would require an excessively deep bore pit for a tenchless crossing. Bore pits at least 25 feet deep would be required, which would require the use of a bench and interin access ramp and create a large volume of material to be excavated and	
NOTOR	H650-027	W-RU29U, W-RU29d, S-RUU1, S-RU29, W-RU20	Conventional Bore	290	26	N	55.2	'	\$747,683.00	Dry-Ditch Open-Cut	stockpiled. Additionally, a conventional bore crossing would extend the duration of this crossing from 10 to 26 days, thereby increasing the potential for an erosion or sedimentation event to occur. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-028	W-A030b W-A030a S-A031	Dry-Ditch Open-Cut	223	-	N	19.8	N	\$122,280.00	Dry-Ditch Open-Cut	This UNT to White Oak Creek and PSS/PEM wetland crossing would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpited. The lack of sufficient space to stockpite the material would further complicate a trenchless or	
NOTOR	H65U-U26	W-AUSUU, W-AUSUU, S-AUSI	Conventional Bore	223	27	, n	19.0	N	\$782,330.00	Біу-Бісп ореп-сас	crossing. Additionally, a conventional bore crossing would extend the duration of this crossing from 3 to 24 days, thereby increasing the potential for an erosion or sedimentation event to occur. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-029	W-A031 (1), S-A033a, S-A032, W-A031 (2), S-A033b	Dry-Ditch Open-Cut	265	-	N	1.4	N	\$137,400.00	Dry-Ditch Open-Cut	The wetland and stream complex crosses a point of intersection in the pipeline and cannot be crossed with a single bore. Using two bores to cross the wetland and stream complex	
		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Conventional Bore		12				\$496,508.00	,	would likely result in temporary impacts to the wetland and stream complex due to the bore pit locations.	
Norfolk	H650-030	W-A033	Dry-Ditch Open-Cut	- 66	-	N	63.9	N	\$23,760.00	Dry-Ditch Open-Cut	This crossing is situated on a long and steep slope that would involve logistically difficult construction conditions and would require an excessively deep bore pit for a trenchless crossing, Avoiding/minimizing this minor impact through a conventional bore would require a bore pit of approximately 24 feet deep. This depth, combined with the severity of the slope	
			Conventional Bore		24				\$370,404.00		on which it would be situated, would exceed the reach of the excavator and require the operator to work from a shallow bench within the pit. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed	
Norfolk	H650-031	W-4035, S-4034, W-4034	Dry-Ditch Open-Cut	- 98	-	N	30.6	N	\$77,280.00	Dry-Ditch Open-Cut	This crossing is situated on a long and steep slope that would involve logistically difficult construction conditions and would require an excessively deep bore pit for a trenchiess crossing, Avoiding/minizing this mism or impact through a conventional bore would require a bore pit at least 35 feet deep. Due to this depth, the use of a bench and interim access ranno would be required, which would create a larse wulner of material to be executed.	
			Conventional Bore		35				\$1,020,005.00	,	and stockpited. Additionally, a conventional bore crossing would extend the duration of this crossing from 9 to 21 days, thereby increasing the noise, aesthetic, and other impacts or nearby persons. Truthermore, the conventional bore crossing cost to avoid the tempor or impacts is unreasonably high relative to the proposed construction method.	
			Dry-Ditch Open-Cut		-				\$125,880.00		Avoiding/minimizing the minor impact to this UNT to Sandy Creek (less than five feet wide) and 0.03 acre of PSS wetfand through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shring and soling plans, which increases the complexity of the excavation. To maintain a safe excavation per CSHA requirements, his sexvantion are given to soing and increase the footprint and spots to the contraction of the contraction o	
Norfolk	H650-032	S-A036, S-A037, W-A036	Conventional Bore	233	23	N	7.4	N	\$637,058.00	Dry-Ditch Open-Cut	requirements, use schemol may require spring and increase the roughing and market all from the roughing and market all from the one pith an already restricted workspeed. Additionally, a conventional bore crossing would extend the duration of this crossing from 10 to 22 days, thereby increasing the potential for an erosion or sedimentation event to occur. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-033	S-A038	Dry-Ditch Open-Cut	42	-	N	10.0	Y	\$57,120.00	Drv-Ditch Open-Cut	This ephemeral stream is a very small (less than five feet in width) UNT to Sandy Creek with little to no observable flow, and open cut would result in minor temporary impacts to this feature. This stream will already be impacted as it is completely within ATWS-1096.	

USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale	
Horiox	11000 000	07000	Conventional Bore	72	18		10.0	·	\$207,464.00	bly bledt open out	Additionally, with the lack of buffer due to the ATWS, a conventional bore crossing would extend the duration of this crossing from 1 to 11 days, further increasing the potential for an erosion or sedimentation event to occur.	
			Dry-Ditch Open-Cut		-				\$59,280.00		There are no significant constraints on available crossing methods or significant	
Norfolk	H650-034	S-A039-Braid1, S-A039	Conventional Bore	48	17	N	8.9	N	\$216,224.00	Conventional Bore	environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
Norfolk	H650-035	W-A038, S-A040, W-A037, S-A041, S-A042	Dry-Ditch Open-Cut	200	-	N	11.6	N	\$114,000.00	. Dry-Ditch Open-Cut	This crossing would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockplied. The lack of sufficient space to stockplie the material would further complicate a tenchless crossing. Additionally, a conventional bore crossing would be set the duration of this crossing from 10 to 24 days, thereby increasing	
			Conventional Bore		28				\$794,425.00		the potential for an erosion or sedimentation even to occur. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-036	W-A039, S-A043	Dry-Ditch Open-Cut	90	-	N	20.9	Y	\$74,400.00	Dry-Ditch Open-Cut	This small UNT to Sandy Creek (less than five feet wide) and PFO wetland crossing would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. The lack of sufficient space to stockpile the material would	
			Conventional Bore		25				\$451,115.00		further complicate a trenchless crossing. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-037	S-A044	Dry-Ditch Open-Cut	38	-	N	13.1	Y	\$55,680.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to the stream through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the	
			Conventional Bore		22			·	\$324,955.00		footprint of the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
			Dry-Ditch Open-Cut		-				\$66,120.00		Avoiding/minimizing the minor impact to the stream through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and stoping plans, which increases the complexity of the excavation. To maintain a safe	
Norfolk	H650-038	W-A041, S-A045	Conventional Bore	67	23	N	18.0	N	\$399,265.00	. Dry-Ditch Open-Cut	excavation per CSHA requirements, this excavation may require sloping and increase the footprint and spoils material from the ore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-039	W-A044, S-A046, W-A045 (1), W-A045 (2)	Dry-Ditch Open-Cut	277	-	N	28.6	N	\$141,720.00	Dry-Ditch Open-Cut	This crossing would require a bore pit that is at least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. The lack of sufficient space to stockpile the material would further complicate a trenchless crossing. Additionally, a conventional bore	
Horiox	11000 000	17,000, 07,000, 17,000 (2)	Conventional Bore	277	30		20.0		\$1,030,129.00	. Biy bilan open out	crossing would extend the duration of this crossing from 10 to 26 days, thereby increasing the noise, aesthetic, and other impacts on nearby persons. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-040	W-A049a, S-A049	Dry-Ditch Open-Cut	172	-	Υ	20.6	Y	\$103,920.00	Dry-Ditch Open-Cut	This crossing presents unique challenges under any method. It would require a bore pit that is at least 30 feet deep, and the bore length is longer than is ideal. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high	
			Conventional Bore		32				\$1,036,698.00		relative to the proposed construction method.	
Norfolk Norfolk	H650-041	W-A048, S-A048	Dry-Ditch Open-Cut	38	-	N	13.1		\$55,680.00	- Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Sandy Creek (less than five feet wide) and 0.01 acre of PEM wetland through a conventional bore would require a deep bore pit of nearly 20 feet. The additional equipment and excess spoil materials would greatly limit the available space in a work area that has already been minimized, which would increase the construction difficulty. The lack of sufficient space to sockpile the material would further	
NOTION	11000-041	erroneus, sroneus	Conventional Bore	30	19		10.1	"	\$229,026.00	. Biy-Ditti Open-Cat	complicate a trenchless crossing. Additionally, a trenchless crossing of this resource would increase the noise, aesthetic, and other impacts on nearby persons due to the operating of heavy equipment for an extended time with a private landowner in close proximity. The open-cut method would also reduce the construction duration near a private drinking water well on the property.	
			Dry-Ditch Open-Cut		-				\$54,240.00		Avoiding/minimizing the minor impact to this UNT to Silver Creek (less than five feet wide) through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the	
Norfolk	H650-042	S-A070	Conventional Bore	34	23	N	13.5	N	\$319,116.00	Dry-Ditch Open-Cut	excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the footprint and spoils material from the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-043	S-A051	Dry-Ditch Open-Cut Conventional Bore	104	- 18	N	5.3	Υ	\$79,440.00 \$311,685.00	. Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
			Dry-Ditch Open-Cut	_	=				\$58,920.00		This small UNIT to Silver Creek (less than five feet wide) would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. The lack of sufficient space to stockpile the material would unterte complicate a tenchless	
Norfolk	H650-044	S-A050	Conventional Bore	47	25	N	17.3	N	\$488,830.00	Dry-Ditch Open-Cut	crossing Additionally, a conventional bore crossing would extend the duration of this crossing from 9 to 14 days, thereby increasing the noise, aesthetic, and other impacts on nearby persons. Truthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	

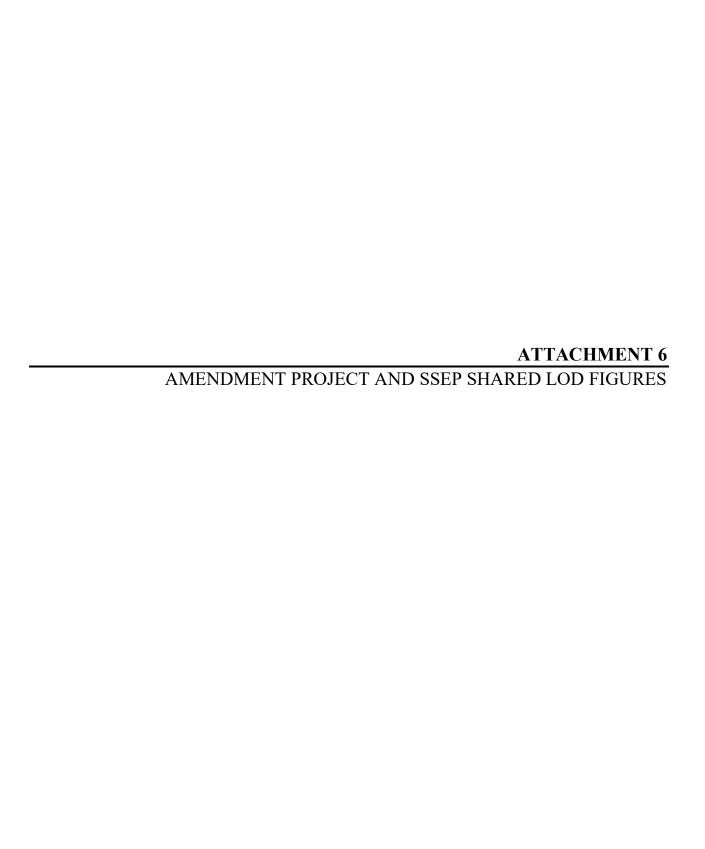
USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale	
Norfolk	H650-045	S-A052	Dry-Ditch Open-Cut	92	-	N	9.0	Y	\$75,120.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Silver Creek (less than five feet wide) through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the topoling of the bore pit in an already restricted workspace.	
			Conventional Bore		21				\$394,660.00		Additionally, a conventional bore crossing would extend the duration of this crossing from b to 1 days, thereby increasing the noise, easthetic, and other impacts on eastly persons. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-046	S-A054, W-A051, S-A055	Dry-Ditch Open-Cut	96	ı	N	16.1	N	\$76,560.00	Dry-Ditch Open-Cut	These small UNT to Silver Creek (each less than five feet wide) and PEM wetland would require a bore pit that is approximately 24 feet deep. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. Or anniatain a safe excavation may require sloping and increases stafe excavation may require sloping and increases.	
Notice		0.1004, 11.1002, 0.1000	Conventional Bore	50	24		10.1		\$578,640.00	,	the footprint of the bore pit in an already restricted workspace. The lack of sufficient space to stockpile the material would stimpter complicate a tenchless crossing. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-047	W-A054, S-A057	Dry-Ditch Open-Cut	134	-	N	4.8	N	\$90,240.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Sandy River (less than five feet wide) and 0.1 acre of PEH wetland through a conventional bore would require a deep bore pit of nearly 20 feet. The additional equipment and excess spoil materials would greatly limit the available space in a work are a but has already been minimized, which would increase the	
Notice	1666 647	111000	Conventional Bore		18		4.0		\$360,052.00	by brain open out	construction difficulty. The lack of sufficient space to stockpile the material would further complicate a trenchless crossing. Additionally, a comentional bore crossing would extend the duration of this crossing from 9 to 17 days, thereby increasing the noise, aesthetic, and other impacts on nearby persons.	
Norfolk	H650-048	S-A058	Dry-Ditch Open-Cut Conventional Bore	- 51	16	N	6.9	N	\$60,360.00 \$216,037.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
			Dry-Ditch Open-Cut		-				\$62,160.00		This UNT to Sandy River would require a bore pit that is at least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a	
Norfolk	H650-049	S-A071	Conventional Bore	56	32	N	28.5	Y	\$931,283.00	Dry-Ditch Open-Cut	large volume of material to be excavated and stockpiled. Furthermore, the conventior bore crossing cost to avoid the temporary impacts is unreasonably high relative to th proposed construction method.	
		S-B059	Dry-Ditch Open-Cut		-				\$56,760.00		This ephemeral stream is a very small (less than five feet in width) UNT to the Sandy River, and open cut would result in minor temporary impacts to this feature. Additionally, a	
Norfolk	H650-055	3-0039	Conventional Bore	41	15	N	7.2	N	\$183,169.00	Dry-Ditch Open-Cut	trenchless crossing of this resource would result in the safety risk of operating heavy equipment for an extended time with a private landowner in close proximity, thereby increasing the noise, aesthetic, and other impacts on nearby persons.	
Norfolk	H650-056	S-B046, W-B043	Dry-Ditch Open-Cut	81	-	N	9.2	Y	\$71,160.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Sandy River (less than five feet wide) and 0.1 acre of PEH wetland through a conventional bore would require a deep bore pit of nearly 201ect. The additional equipment and excess spoil materials would greatly limit the available space in a work are a that has already been minimized, which would increase the	
			Conventional Bore		19				\$278,105.00		construction difficulty. Additionally, a trenchless crossing of this resource would result the safety risk of operating heavy equipment for an extended time with a private landow in close proximity, thereby increasing the noise, aesthetic, and other impacts on near persons.	
Norfolk	H650-057	W-B042, S-B045	Dry-Ditch Open-Cut	148	1	N	37.4	Y	\$95,280.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact to one PEMPSS wetland and UNT to Trayner Branch (less than five feet wide). This crossing is situated on a steep slope that would involve logistically difficult construction conditions and would require an excessively deep bore pit for a Tenrichless crossing. Bore pits at least 25 feet deep would be required.	
			Conventional Bore		27			·	\$691,100.00	2,7 2,331 2,231 2,231	which would create a large volume of material to be excavated and stockpited. Due to this depth, the use of a bench and interim access ramp would be also be required. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-058	W-8041. \$-8044a	Dry-Ditch Open-Cut	48	-	N	13.6	v	\$59,280.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Trayner Branch (less than five feet wide) and 0.01 acre of PSS wetland through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA	
NOTOR	11030-030	W-0041, 3-00446	Conventional Bore	40	22		15.0	'	\$348,689.00	, ,	requirements, this excavation may require sloping and increase the footprint of the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-059	S-B043, W-B039b	Dry-Ditch Open-Cut Conventional Bore	132	- 20	N	9.6	Y	\$89,520.00 \$366,266.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/iminimized by use of the conventional bore method.	
			Dry-Ditch Open-Cut		-				\$131,640.00		This crossing would require a bore pit that is at least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume	
Norfolk	H650-060	W-B038a, S-B041, S-B042, W-B038b, W-B038c	Conventional Bore	249 32	N	13.3	Y	\$1,039,488.00	Dry-Ditch Open-Cut	of material to be excavated and stockpiled. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.		
Norfolk	H650-060B	W-F002b	Dry-Ditch Open-Cut	139	-	N	8.8	Y	\$50,040.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of 0.02 acre of PFO wetland. A conventional bore crossing would extend the duration of this crossing from 3 to 15 days. Thereby increasing the noise, aesthete, and other impacts no nearby persons. Futthermore,	
			Conventional Bore		15				\$335,383.00		the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	

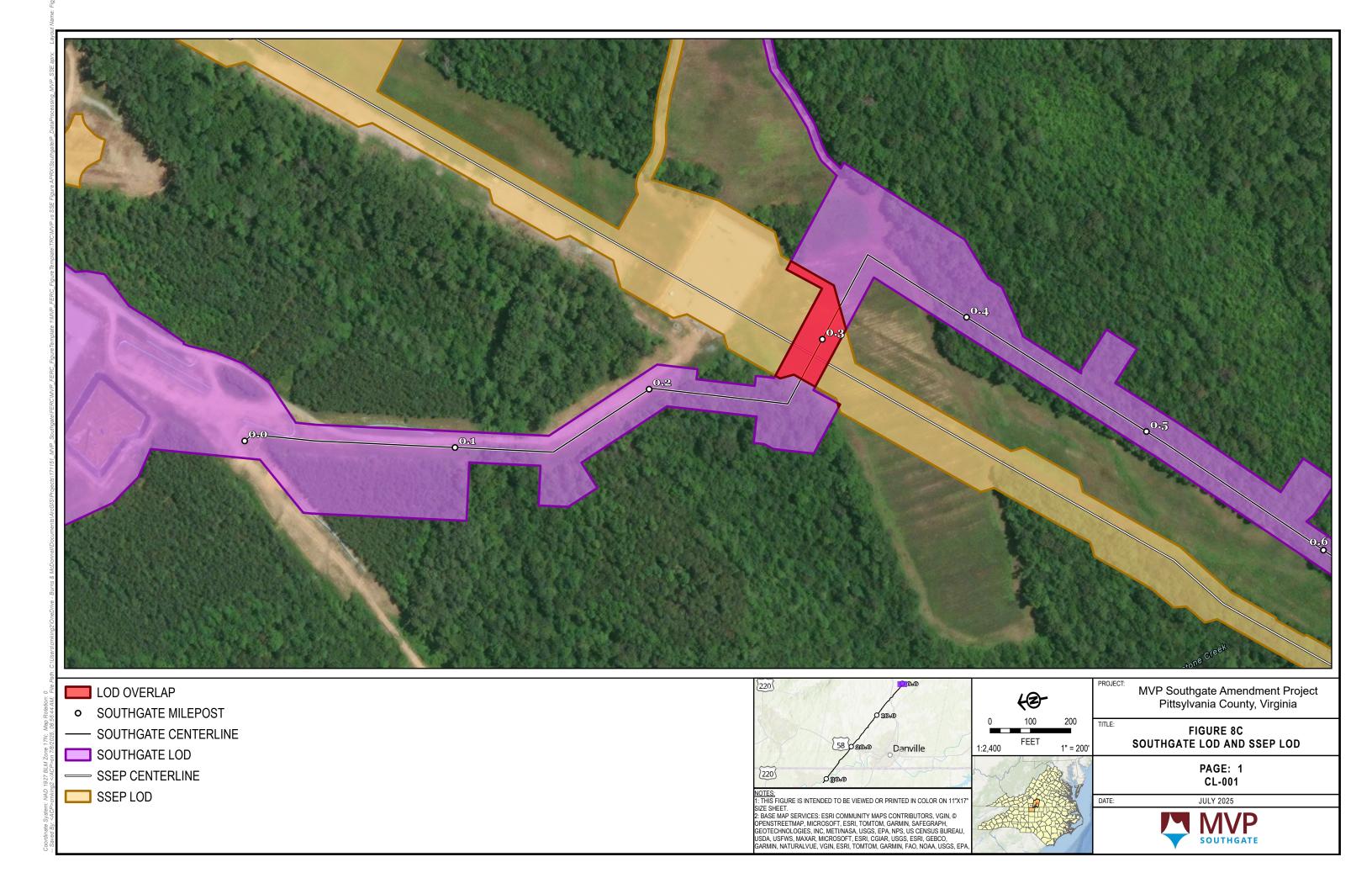
USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale	
Norfolk	H650-061	W-B036a, S-B040	Dry-Ditch Open-Cut	93	-	N	19.4	N	\$75,480.00	. Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Trotters Creek (less than five feet wide) and 0.02 acre of PFO wetland through a conventional bore would require a deep bore pit of approximately 2 Heef. This depth would require engineered shoring and sloping plans, which increases the complexity of the exzavation. To maintain a safe exzavation per OSHA requirements, his exzavation may require sloping and increase the forbigina day spots.	
			Conventional Bore		24				\$400,689.00		from the bore jit in an already existiced workspace. Furthermore, the conventional borc from the bore jit in an already existiced workspace. Furthermore, the conventional borc crossing cost to avoid the temporary impacts is unreasonably high relative to the propose construction method.	
Norfolk	H650-062	W-B036b	Dry-Ditch Open-Cut Conventional Bore	- 99	- 13	N	4.6	Y	\$35,640.00 \$258.715.00	. Conventional Bore	This wetland will be bored in conjunction with C.R. 875. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
			Dry-Ditch Open-Cut		-				\$84,480.00		Avoiding/minimizing the minor impact to this UNT to Trotters Creek (less than five feet wide) and 0.03 acre of PFO wetland through a conventional bore would require a bore pit that is at	
Norfolk	H650-062B	S-B032, W-B023	Conventional Bore	118	34	N	25.6	N	\$852,797.00	Dry-Ditch Open-Cut	least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
			Dry-Ditch Open-Cut		-				\$55,320.00		Avoiding/minimizing the minor impact to this UNT to Trotters Creek (less than five feet wide) and 0.01 acre of PEM wetland through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the combestiv of the excavation. To maintain a safe execution per OSHA	
Norfolk	H650-063	S-B033, W-B024	Conventional Bore	37	21	N	11.7	N	\$323,494.00	Dry-Ditch Open-Cut	includes selection (people) on the execution is of interested as an execution per CSPAN requirements, his securation may require sloping and recease the frostprint and spoiss material from the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to would the temporary impact is unreasonably high relative to the proposed construction method.	
No. of the	11050 004	0.000	Dry-Ditch Open-Cut		-		20.4		\$65,760.00	D. D	This ephemeral UNT to Trotters Creek would require a bore pit that is at least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume of material to be excavated and stockpiled. The lack of	
Norfolk	H650-064	S-B039	Conventional Bore	- 66	31	N	20.1	N	\$891,074.00	Dry-Ditch Open-Cut	sufficient space to stockpile the material would further complicate a trenchless crossing. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-065	S-B061	Dry-Ditch Open-Cut	- 56	-	N N	12.4	N	\$62,160.00	Dry-Ditch Open-Cut	Avoiding/minimizing the minor impact to this UNT to Trotters Creek (less than five feet wide) through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation maintain a safe excavation per OSHA requirements, this excavation per	
			Conventional Bore		21				\$337,533.00		require sloping and increase the footprint and spoils material from the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-066	S-B029	Dry-Ditch Open-Cut	- 57	-	N	45.2	N	\$62,520.00	Dry-Ditch Open-Cut	This crossing is situated on a steep slope that would involve logistically difficult construction conditions and would require an excessive by deep bore git for a trenchless crossing. Bore pits at least 30 feet deep would be required, which would create a large volume of material to be excavated and stockpiled. Due to this depth, the use of a bench	
			Conventional Bore		33				\$951,012.00		and interim access ramp would be also be required. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method. There are no significant constraints on available crossing methods or significant.	
Norfolk	H650-067	W-B022, S-B031, S-B030	Dry-Ditch Open-Cut Conventional Bore	99	20	N	11.4	N	\$77,640.00 \$299,818.00	Conventional Bore	environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
Norfolk	H650-068	S-B024, S-B025	Dry-Ditch Open-Cut Conventional Bore	- 44	16	N	9.3	N	\$57,840.00 \$196,683.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method. There are no significant constraints on available crossing methods or significant	
Norfolk	H650-069	W-B049, S-B022	Dry-Ditch Open-Cut Conventional Bore	103	15	N	7.3	N	\$79,080.00 \$278,256.00	Conventional Bore	environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.	
Norfolk	H650-070	W-B020	Dry-Ditch Open-Cut	121	÷	N	19.7	N	\$43,560.00	. Dry-Ditch Open-Cut	Avoiding/minimizing this temporary impact to 0.12 acre of PSS through a conventional bore would require a bore pit nearly 25 feet deep. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the	
			Conventional Bore		23				\$459,837.00		footprint and spoils material from the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
			Dry-Ditch Open-Cut		-				\$42,120.00		The open-cut method would result in a temporary impact of 0.07 acre of PEM wetland. Avoiding/minimizing this temporary impact to 0.07 acre of PEM wetland through a conventional bore would require a bore pit at least 20 feet deep. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To	
Norfolk	H650-071	W-B019 (1)	Conventional Bore	117	20	N	16.7	N	\$280,428.00	Dry-Ditch Open-Cut	maintain a sale excavation per OSHA requirements, this excavation may require sloping and increase the footprint and spoils material from the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	
Norfolk	H650-072A	W-B019 (2)	Dry-Ditch Open-Cut	79	÷	N	9.2	N	\$28,440.00	.00 conventional bore would require a b sufficient space to stockpile the mater	Avoiding/minimizing this temporary impact to 0.04 acre of PEM wetland through a conventional bore would require a bore pit of approximately 17 feet deep. The lack of sufficient space to stockpile the material would further complicate a trenchless crossing. The reasons of crobbles in the surface so all so in directors contentially comblematic	
m T 1/0/05		– Jav (L)	Conventional Bore	35	17		9.2	N	\$224,948.00	_,	The presence of cobbies in the surface soil also indicates potentially problematic subsurface conditions for conventional auger bore machines. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.	

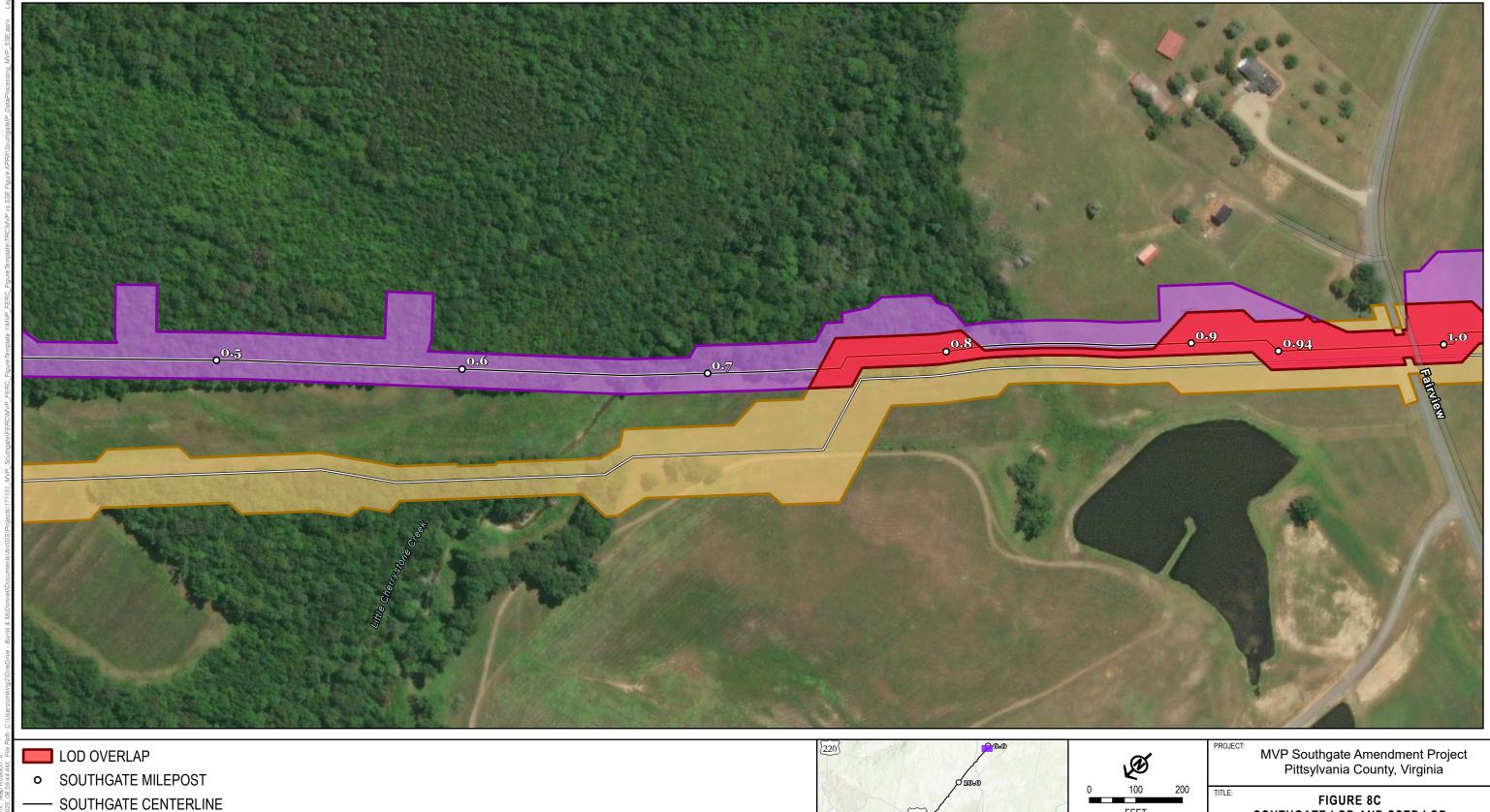
USACE District	Crossing #	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale
Norfolk	H650-072B	S-B056	Dry-Ditch Open-Cut	- 56	-	N	10.7	N	\$62,160.00	Dry-Ditch Open-Cut	This ephemeral stream is a very small (less than five feet in width) UNT to Sandy Creek, and open cut would result in minor temporary impacts to this feature. Avoiding/minimizing this minor impact through a conventional hore would require a bore plat nearly 20 feet deep. This depth would require engineered shoring and sloping plans, which increases the complexity of the exavation. To minimize a set execution nor SOHA requirements, this
			Conventional Bore		19				\$237,038.00		excavation may require sloping and increase the footprint and spoils material from the bore pit in an already restricted workspace. Furthermore, the presence of cobbles in the surface soil indicates potentially problematic subsurface conditions for conventional auger bore machines.
Norfolk	H650-074	W-8017 (1)	Dry-Ditch Open-Cut	145	-	N	15.0	N	\$52,200.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.15 acre of a PEM wetland. The configuration of the right-of-way at this crossing includes a 90-degree point of intersection that would not allow for enough space to establish a bore pit while
North	11000 074	W 5027 (2)	Conventional Bore	1	11		15.0	·	\$298,473.00	biy bikan opan oak	avoiding impacts to nearby wetlands. Furthermore, avoiding/minimizing the minor impact through a conventional bore is an unreasonably high cost relative to the proposed construction method.
Norfolk	H650-075	W-B017 (2)	Dry-Ditch Open-Cut	122	-	N	22	N	\$43,920.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.15 acre of a PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is an
Horiox	1100 070	W 5527 (2)	Conventional Bore		13		2.2		\$278,594.00	Diy Dictiopen out	unreasonably high cost relative to the proposed construction method.
Norfolk	H650-076	S-B054	Dry-Ditch Open-Cut Conventional Bore	- 64	30	- N	21.9	N	\$65,040.00 \$705,448.00	Dry-Ditch Open-Cut	This small UNT to Dan River (less than five feet wide) would require a bore pit that is at least 30 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a large volume or material to be excavated and stockpiled. The lack of sufficient space to stockpile the material would further complicate a trenchless crossing. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
			Dry-Ditch Open-Cut		-				\$63,600.00		The open-cut method would result in a temporary impact to an UNT to the Dan River and less than 0.01 acre of a PFO wetland. Avoiding/minimizing this minor impact through a conventional bore would require a bone pit at least 20 test deep. This depth would require
Norfolk	H650-078	S-8052, W-8015	Conventional Bore	- 60	21	N	18.9	Y	\$471,270.00	Dry-Ditch Open-Cut	engineered shoring and sloping plans, which increases the complexity of the excavation maintain as alse excavation per CSHA requirements, this excavation may require plan and increase the footprint of the bore pit in an already restricted workspace. Furthermor the conventional bore crossing cost to avoid the temporary impacts is unreasonably hi relative to the proposed construction method.
			Dry-Ditch Open-Cut		-				\$62,520.00		Avoiding/minimizing the minor impact to this UNT to the Dan River (less than five feet wide) through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To ministina is a side excavation per OSH-feety immens, this secretarion may require sloping and increase the footprint of the bore pit in an aiready restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Norfolk	H650-079	S-B051	Conventional Bore	57	21	N	10.2	Y	\$352,694.00	Dry-Ditch Open-Cut	
Norfolk	H650-080	W-B014 (1), S-B020, W-B014 (2)	Dry-Ditch Open-Cut Conventional Bore	- 82	16	N	10.7	Υ	\$71,520.00 \$270,431.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
Norfolk	H650-081	W-B013b	Dry-Ditch Open-Cut	174	-	N	6.4	Y	\$62,640.00	Dry-Ditch Open-Cut	Avoiding/minimizing this minor impact through a conventional bore crossing would extend the duration of this crossing from 3 to 16 days, thereby increasing the potential for an erosion or sedimentation event to occur. Furthermore, the conventional bore crossing cost
TOTOR	11000 002	** 50105	Conventional Bore	274	16	.,	0.4		\$391,050.00	biy bikin open out	to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Wilmington	H650-082	W-B012	Dry-Ditch Open-Cut	148	-	N	3.6	N	\$53,280.00	. Conventional Bore	This wetland will be bored in conjunction with Buffalo Road. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
			Conventional Bore		10				\$298,286.00		be avoiced/minimized by use of the conventional bore method.
Wilmington	H650-083	W-8011	Dry-Ditch Open-Cut	46	-	N	9.4	N	\$58,560.00	Dry-Ditch Open-Cut	The open-cut method would result in minor temporary impacts of 0.03 acre to a PEM wetland. In order to complete this bore without impacts to other wetlands in close proximity (W-F003, S-8018, S-8019), the centerline would need to be offset. This would require additional welds that are not contemplated in the standard base lay oss, especially since
···aingon			Conventional Bore	40	15	N	3.4	,	\$181,335.00	υτy-υπαπ Open-Cut	acontrol weets that are not contempated in the standard base yous, especially since this crossing is between two road bones that are effectively set in their locations. Additionally, avoiding impacts to those other resources requires further reduces the available workspace for stockpling material from the bore pit excavation.
Wilmington	H650-083B	W-F006	Dry-Ditch Open-Cut	134	-	N	5.0	N	\$48,240.00	. Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be
<u> </u>		***	Conventional Bore		18				\$328,083.00		environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
Wilmineton	H650-084	W-R032 S-F005	Dry-Ditch Open-Cut	120	-	N N	15.6	Y	\$43,200.00	Drv-Ditch Onen-Cut	Avoiding/minimizing this temporary impact to an UNT of Cascade Creek and 0.11 acre of PEH weetland through a conventional bore would require a relatively deep bore pit more than 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity of the excavation. To maintain a safe excavation per OSHA continuement the law-evaluation must remise distributed informace the forbinding of the hove not

USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale
Wannigon	1100 004	W 5002,0 1 000	Conventional Bore	110	22		2000		\$435,541.00	bry bremopen dat	in an already restricted workspace. Additionally, a conventional bore crossing would extend the duration of this crossing from 9 to 15 days, thereby increasing the noise, assthetic, and other impacts on early persons. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Wilmington	H650-085A	W-B031b, W-B031a	Dry-Ditch Open-Cut	305	4	N	0.0	٧	\$109,800.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.5 acre of PEM and PSS wetlands. The duration of the trenchless crossing is over 7 times longer than the open-cut process due to only crossing wetlands, thereby increasing the noise, aesthetic, and other impacts on nearby persons. Additionally, the crossing length exceeds 200 feet at
Thum, got	11000 0000	W 50015, W 50012	Conventional Bore	535	17		0.0	·	\$710,208.00	Dry Dictiropen out	this location. Reducing the time at the crossing and permanently stabilizing this area will reduce the potential for sedimentation and erosion. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Wilmington	H650-085B	W-B028 (1)	Dry-Ditch Open-Cut	36	-	N	2.5	N	\$12,960.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.1 acre of a PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is an
			Conventional Bore		10				\$143,900.00		unreasonably high cost relative to the proposed construction method.
Miles	H650-085C	W-8028 (2)	Dry-Ditch Open-Cut	287	-	N	1.3	N	\$103,320.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.4 acre of PEM wetland. Because this wetland complex is so large, it would have to be trammed across in order to get equipment in to set up bore pits and stage pipe for a bore crossing. Additionally, the size of the wetland complex indicates this ground is saturated which would require
Wilmington	H650-085C	W-BU28 (2)	Conventional Bore	28/	9	N	1.3	N	\$505,793.00	Dry-Ditch Open-Cut	substantial continuous dewatering operations in the bore pits. A conventional bore crossing would extend the duration of this crossing from 3 to 15 days, thereby increasing the noise, aesthetic, and other impacts on nearby persons. The lack of sufficient space to stockpile the material would further complicate a trenchless crossing.
Wilmington	H650-086A	W-B028 (3), W-B056a	Dry-Ditch Open-Cut	241	-	N	0.0	N	\$86,760.00	Dry-Ditch Open-Cut	Avoiding/minimizing the temporary impact to 0.3 acre of PEM wetland and 0.05 acre of PSS wetland through a conventional bore would extend the duration of this crossing from 3 to 17 days, thereby increasing the noise, ae
Thum, But	11000 0001	5020 (5), 50003	Conventional Bore	2-72	9		0.0		\$434,066.00	Diy Dicir Open Gut	Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Wilmington	H650-086B	W-B027, S-B036	Dry-Ditch Open-Cut Conventional Bore	101	19	N	2.6	N	\$78,360.00 \$302,738.00	Conventional Bore	There are no significant constraints on available crossing methods or significant environmental impacts relevant to the available methods. The direct aquatic impact will be avoided/minimized by use of the conventional bore method.
Wilmington	H650-087B	W-F013	Dry-Ditch Open-Cut	52	-	N	0.7	Υ	\$18,720.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.03 acre of a PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is an
			Conventional Bore		9				\$158,126.00	,	unreasonably high cost relative to the proposed construction method.
Wilmington	H650-087C	W-F009	Dry-Ditch Open-Cut	315	-	N	0.0	Y	\$118,800.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.55 acre of a PEM wetland. The duration of the trenchless crossing is 7 times longer than the open-cut process due to only crossing a wetland, thereby increasing the noise, aesthetic, and other impacts on nearby persons. Additionally, the crossing length exceeds 300 feet at this
			Conventional Bore		11				\$481,800.00		location. Reducing the time at the crossing and permanently stabilizing this area will reduce the potential for sedimentation and erosion. The length of the bore also exceeds the limits of the equipment needed for a conventional bore.
Wilmington	H650-088	W-F011, W-F012, W-B010	Dry-Ditch Open-Cut Conventional Bore	225	- 11	N	2.0	Υ	\$81,000.00 \$415,273.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.09 acre of a PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is an unreasonably high cost relative to the proposed construction method.
			Dry-Ditch Open-Cut		-				\$44,640.00		The open-cut method would result in a temporary impact of approximately 0.05 acre of a
Wilmington	H650-089	W-B009b	Conventional Bore	124	14	N	1.9	Y	\$286,081.00	Dry-Ditch Open-Cut	PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is unreasonably high relative to the proposed construction method.
Wilmington	H650-090	W-B009a, S-B015	Dry-Ditch Open-Cut	274	-	N	11.1		\$140,640.00	Dry-Ditch Open-Cut	Avoiding/minimizing the temporary impact to this UNT to the Dan River and 0.14 acre of PFO wetland through a conventional bore would require a deep bore pit of over 20 feet. This depth would require engineered shoring and sloping plans, which increases the complexity
waningun	1650-090	W-00094, 3-8019	Conventional Bore	2/4	21		11.1	,	\$783,710.00	Diy-Ditch Open-Cut	of the excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloping and increase the footprint of the bore pit in an already restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
			Dry-Ditch Open-Cut		-				\$53,520.00		Avoiding the temporary impact to this UNT to the Dan River through a conventional bore would require moving bore pits to the eastern edge of the permanent easement as the
Wilmington	H650-091	S-B016	Conventional Bore	32	10	N	4.9	N	\$128,926.00	Dry-Ditch Open-Cut	stream ends before crossing the centerline. Due to the stream terminating before crossing the centerline. Due to the stream terminating before crossing the centerline, this feature will likely be avoided during the construction of this crossing.
Wilmington	H650-092	W-8008, S-B017	Dry-Ditch Open-Cut	46	-	N	16.4	•	\$58,560.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of 0.01 acre of PEM wetland and an UNT to the Dan River (less than five feet wide). Avoiding/minimizing this minor impact through a conventional bore would require a bore pt at least 20 feet deep. This depth would require engineered shoring and slooping plans, which increases the complexity of the
			Conventional Bore		21			· 	\$332,067.00	z., zopen out	excavation. To maintain a safe excavation per OSHA requirements, this excavation may require sloging and increase the footborn of the bore pit in an arealy restricted workspace. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Wilmington	H650-093	S-B011	Dry-Ditch Open-Cut	52	-	N N	21.0	γ	\$60,720.00	Drv-Ditch Open-Cut	This small UNT to Dan River (less than five feet wide) would require a bore pit that is at least 25 feet deep. Due to this depth, the use of a bench and interim access ramp would be required, which would create a larse volume of material to be excavated and stocknilled.

USACE District	Crossing#	Waterbody	Crossing Methods Evaluated	Crossing Length	Pit Depth	Deep Stream	Maximum Average Slope (%)	Sufficient Stockpile Storage Available	Total Cost (\$)	Proposed Crossing Method	Crossing Method Decision Rationale
Walmigton	11000 000	0 0011	Conventional Bore	02	25		27.0	·	\$377,367.00	Dry Dictir Open Out	Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
Wilmington	H650-094	W-B052a, W-B052b	Dry-Ditch Open-Cut	65	-	N	9.3	Y	\$23,400.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.03 acre of PFO wetland. Avoiding/minimizing the minor impact through a conventional bore is
			Conventional Bore		15				\$218,209.00	,	unreasonably high relative to the proposed construction method.
Wilmington	H650-095 S-B009	Dry-Ditch Open-Cut	31	-	N 351	35.1		\$53,160.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact to one ephemeral UNT to the Dan River (less than five feet wide). This crossing is also situated on a long and steep slope that would imove logistically difficult construction conditions and would require an excessively deep bore git for a trenches crossing. Bore gits at least 35 feet deep would be required.	
			Conventional Bore	-	35			·	\$885,645.00		which would necessitate the use of a bench and interim access ramp and create a large volume of material to be excavated and stockpiled. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
			Dry-Ditch Open-Cut		-				\$58,200.00		Impacts to this stream are required as it is within the pull back area for the Dan River HDD. Additionally, a trenchless crossing of this resource would result in an extended time with a
Wilmington	H650-096	S-B008		45		N	1.8	Y		Dry-Ditch Open-Cut	routiness of usange of usange in close proximity, thereby increasing the noise, aesthetic, and other impacts on nearby persons. Furthermore, surface soil data indicates that the groundward reviel at this location is high and that the soil is frequently flooded. The bore pits would be within the 100-year floodplain and constant bore pit dewatering activities would be required to complete the trenchless crossing.
			Conventional Bore		12				\$175,308.00	biy-bitch Open-Cut	
Wilmington	H650-097	W-8005	Dry-Ditch Open-Cut	1020	-	N	0.0		\$367,200.00	Dry-Ditch Open-Cut	Impacts to this wetland are required as it is within the pull back area for the Dan River HDD. Inclusion of this wetland in the HDD is not feasible without rerouting the pipeline. The
waningan	11030-037	V-5005	Conventional Bore	1020	15	, and the second	0.0	·	\$1,489,200.00	Diy-Ditch Open-Cut	crossing is too long and exceeds capabilities of conventional bore machines, and the configuration of the LOD does not allow for non-conventional techniques without impacting other wetlands and streams. Therefore the conventional bore method is not feasible.
Wilmington	H650-099	W-B056	Dry-Ditch Open-Cut	79	-	N	1.9	Y	\$28,440.00	Dry-Ditch Open-Cut	The open-cut method would result in a temporary impact of approximately 0.04 acre of a PEM wetland. Avoiding/minimizing the minor impact through a conventional bore is
-		***	Conventional Bore		10				\$206,680.00	,	unreasonably high relative to the proposed construction method.
Wilmington	H650-100	W-B002, S-B002	Dry-Ditch Open-Cut	350	=	N	0.0	Y	\$168,000.00	Dry-Ditch Open-Cut	The trenchless crossing method would not be practicable because the length of bore required to avoid wetland impacts exceeds the limitations of the technology. The upland areas between this wetland are not large enough to accommodate a bore pit, and are not accessible without some kind of wetland impacts. If the entire complex were to be bored, it
			Conventional Bore		10				\$602,340.00	у окупленоренов	would complicate the tie-in to the receiver at the Dan River Interconnect since the bore would be situated deeper than typical pipeline construction. Furthermore, a trenchless crossing would extend the duration of this crossing from 9 to 22 days, thereby increasing the noise, aesthetic, and other impacts on nearby persons.
Wilmington	H650-101	W-8002	Dry-Ditch Open-Cut	93	-	N	1.9	Y	\$33,480.00	Dry-Ditch Open-Cut	Avoiding/minimizing the temporary impact to 0.4 acre of PEM wetland through a conventional bore would extend the duration of this crossing with a private landowner in close proximity, thereby increasing the noise, assisted, and other impacts on nearby persons. Furthermore, the conventional bore crossing cost to avoid the temporary impacts is unreasonably high relative to the proposed construction method.
			Conventional Bore		10				\$227,120.00	.,	







FEET

1:2,400

Danville

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GARMIN, NATURALYUE, VGIN, ESRI, TOMTOM, GARMIN, FAO, NOAA, USGS, EPA,

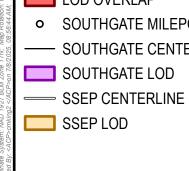
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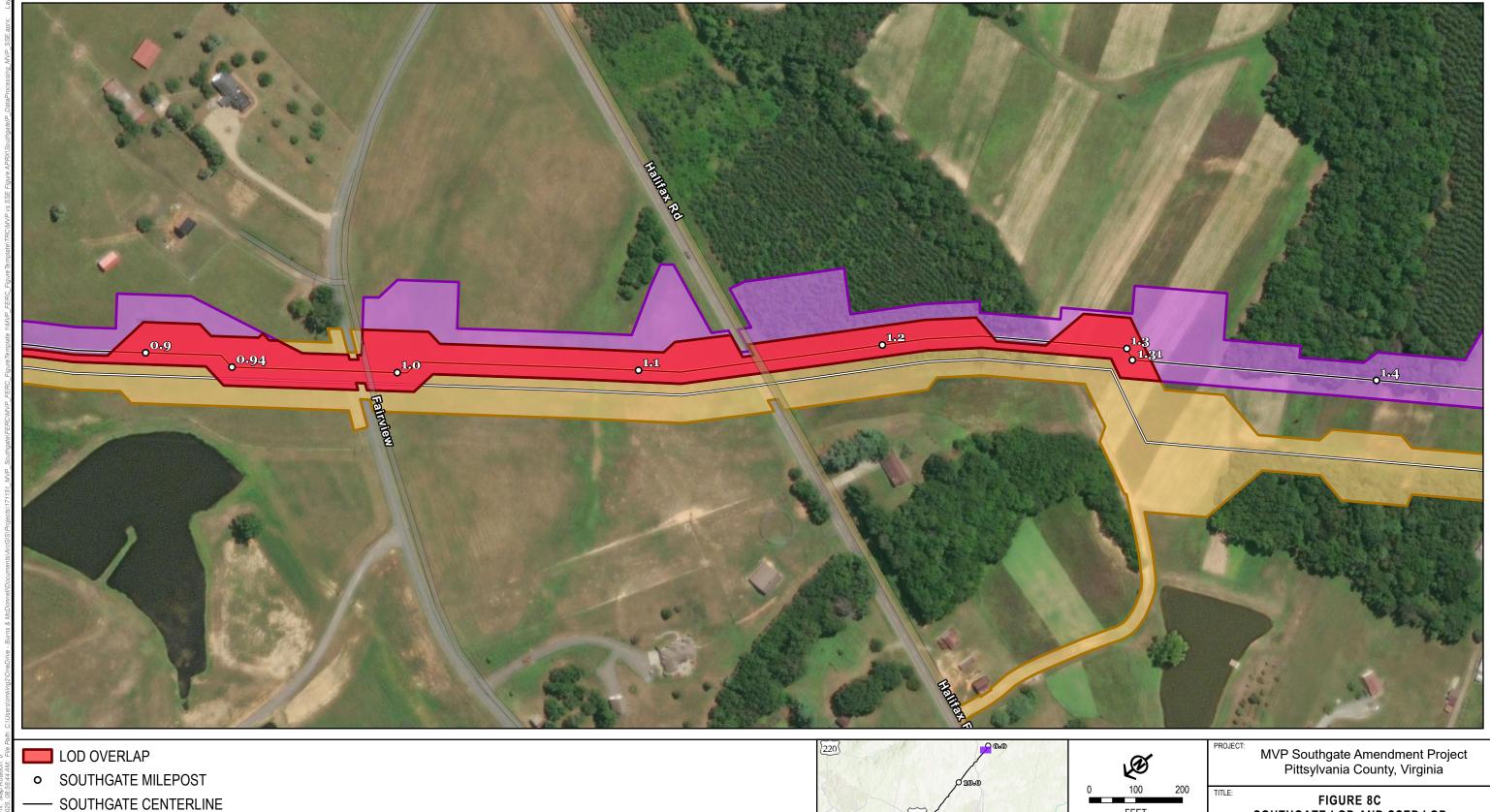
1" = 200'

SOUTHGATE LOD AND SSEP LOD

PAGE: 2

CL-002





FEET

1:2,400

Danville

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GARMIN, NATURALYUE, VGIN, ESRI, TOMTOM, GARMIN, FAO, NOAA, USGS, EPA,

2205

1" = 200'

SOUTHGATE LOD AND SSEP LOD

PAGE: 3

CL-003

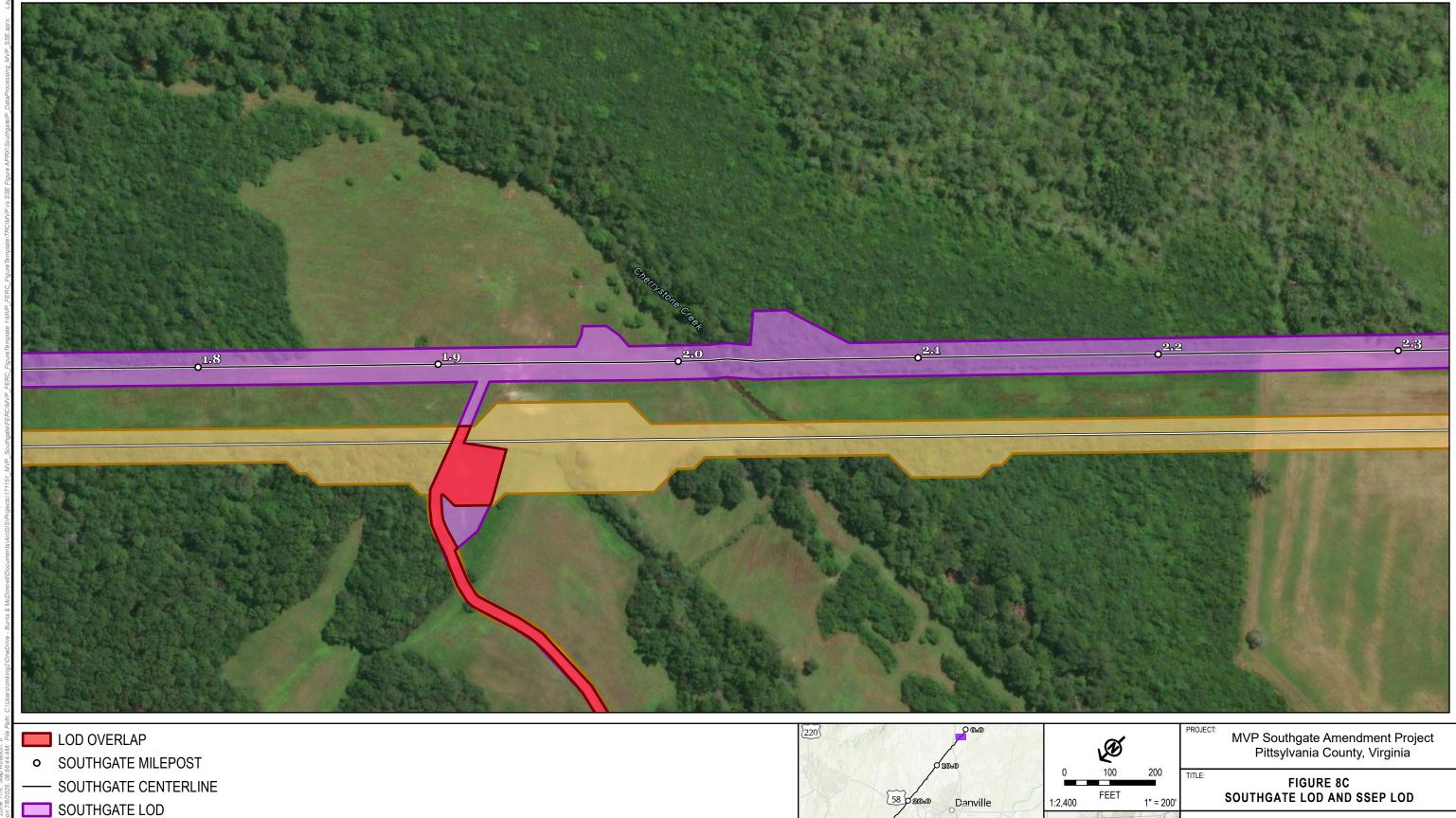
JULY 2025



SOUTHGATE LOD

—— SSEP CENTERLINE

SSEP LOD



220

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PAGE: 4

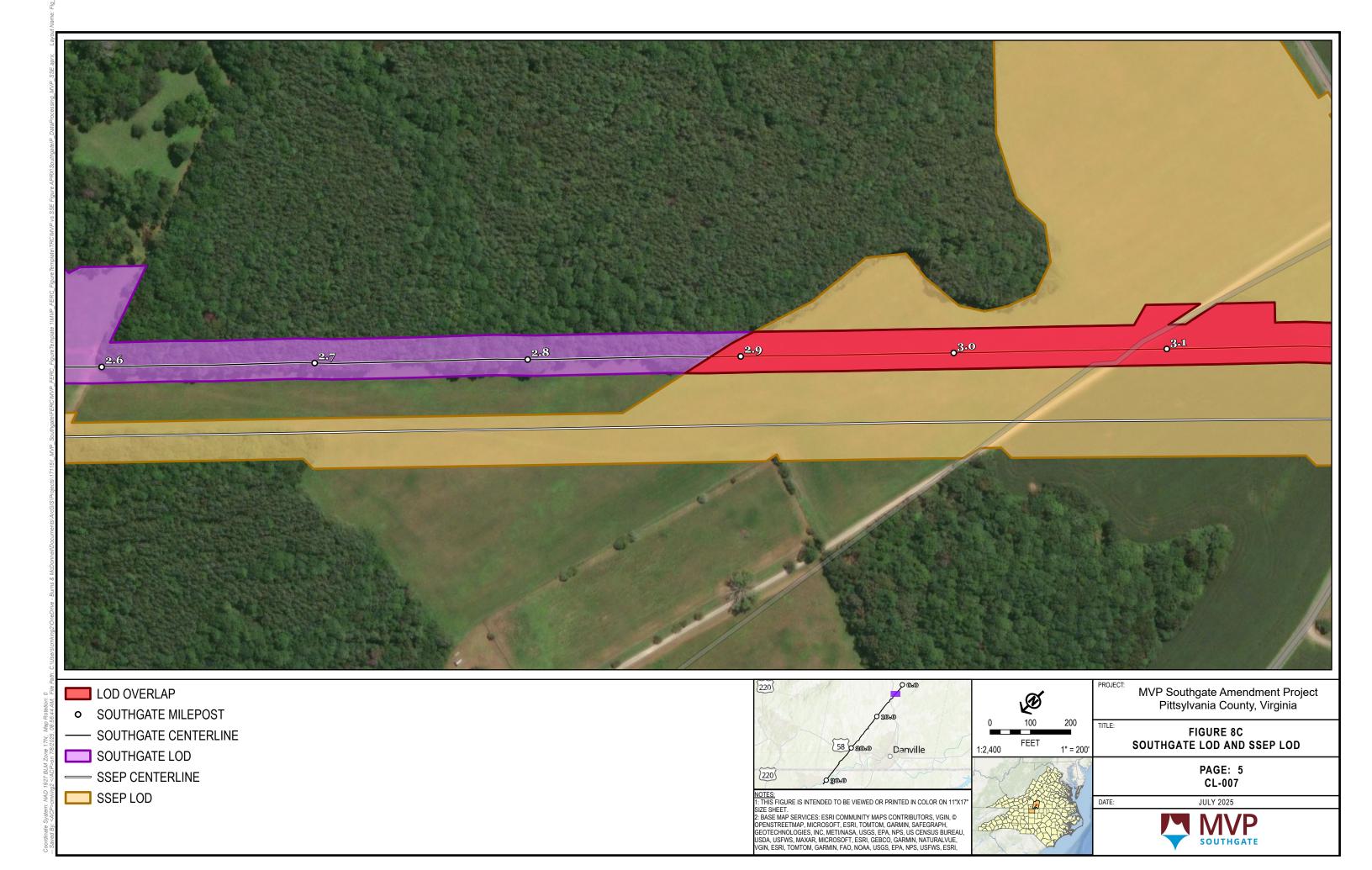
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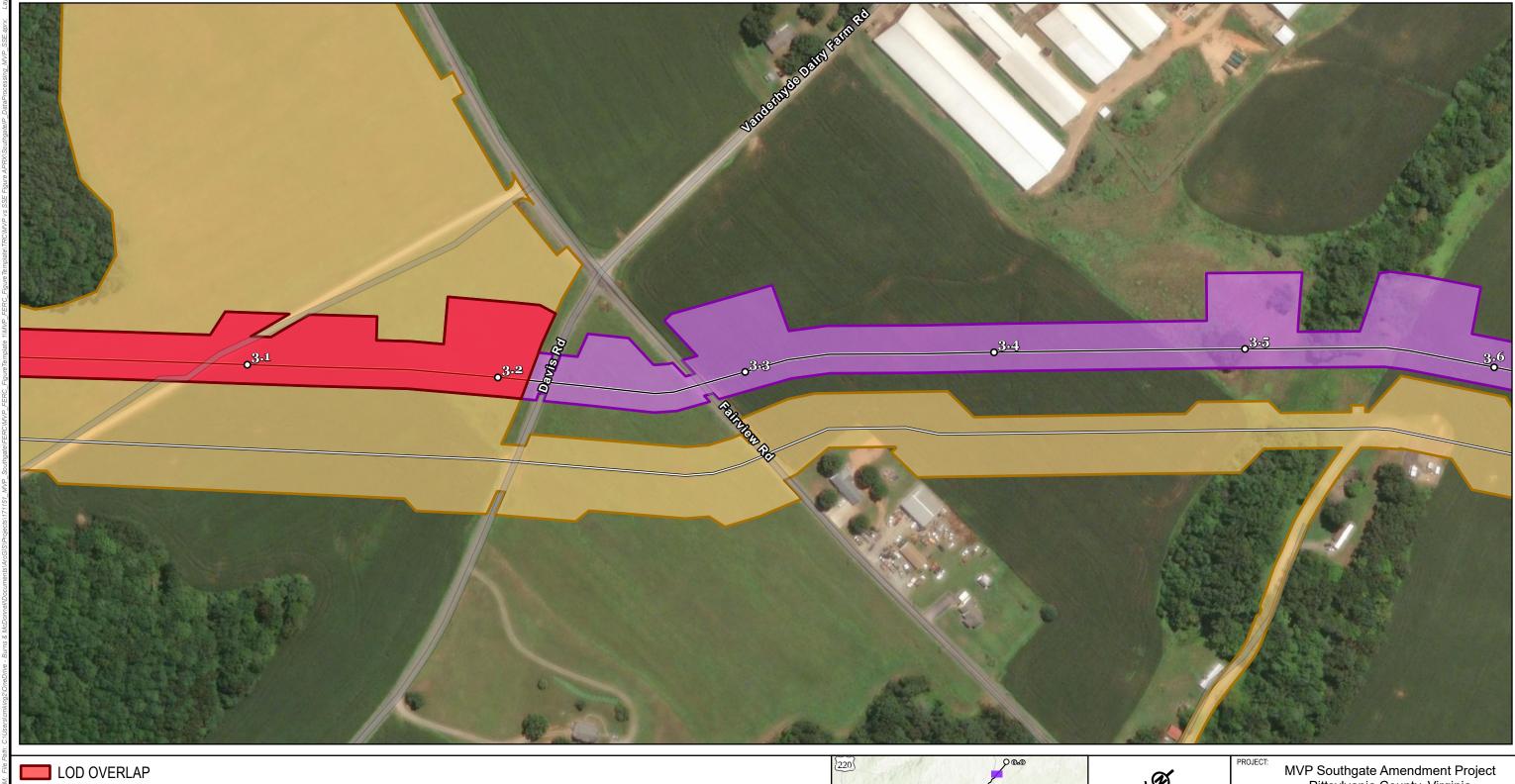
JULY 2025

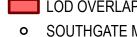
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—— SSEP CENTERLINE

SSEP LOD







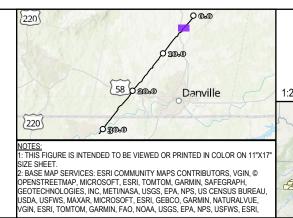
SOUTHGATE MILEPOST

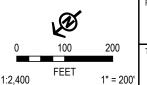
- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD



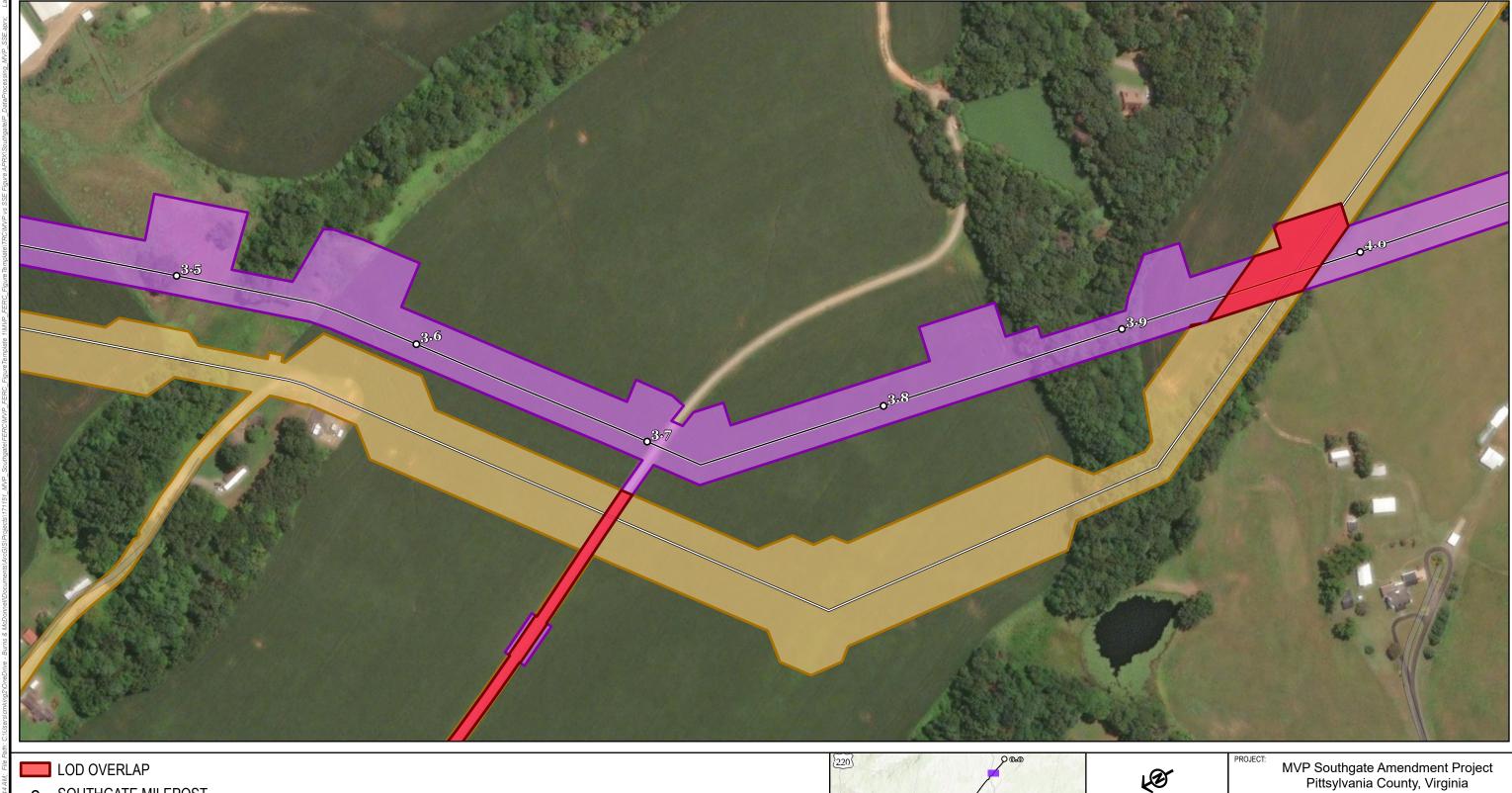


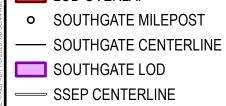
MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 6 CL-008







SSEP LOD

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2205

200

1" = 200'

FEET

Danville

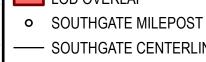
FIGURE 8C

SOUTHGATE LOD AND SSEP LOD

PAGE: 7

CL-009



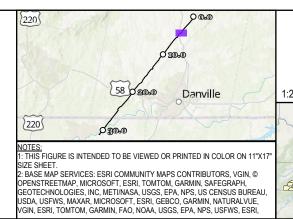


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD



FEET

200

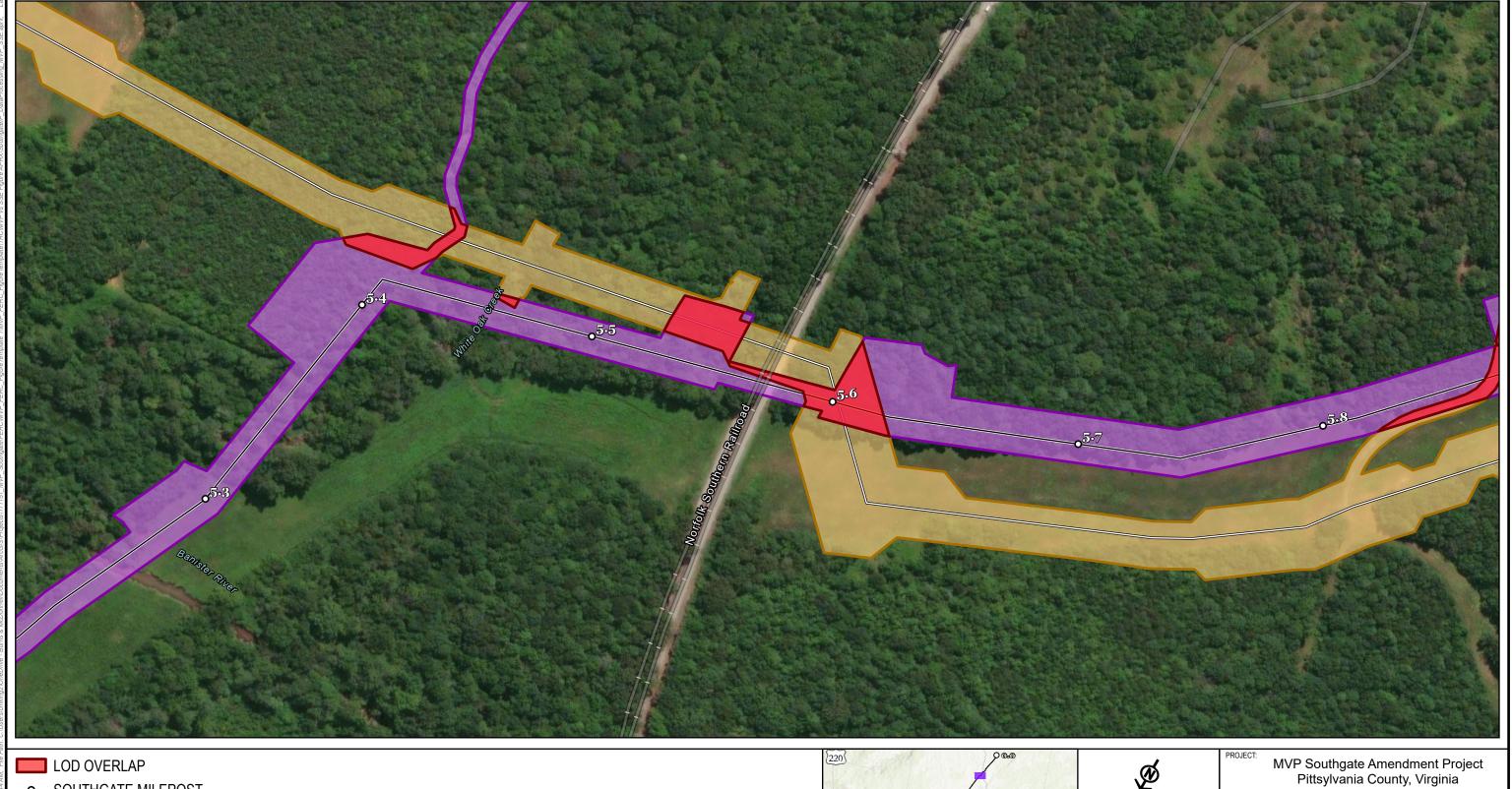
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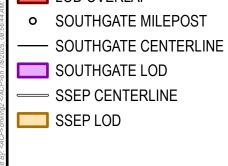
MVP Southgate Amendment Project Pittsylvania County, Virginia

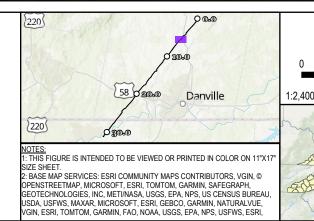
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 8 CL-010











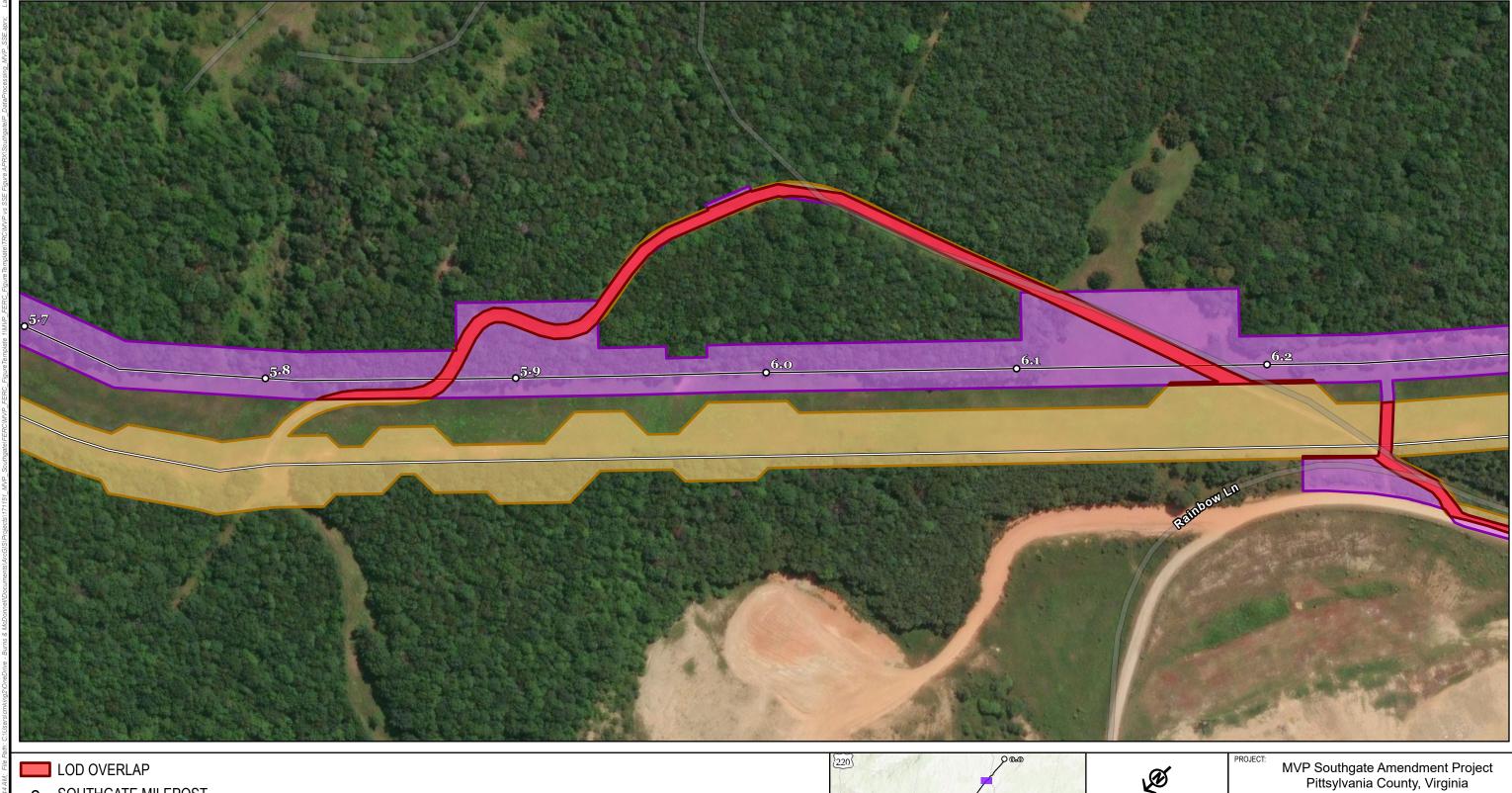
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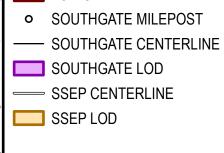
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

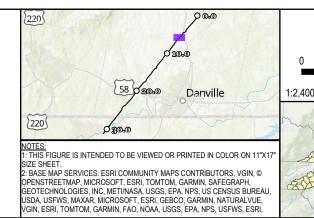
> PAGE: 9 CL-013

: JULY 2025









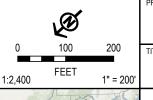


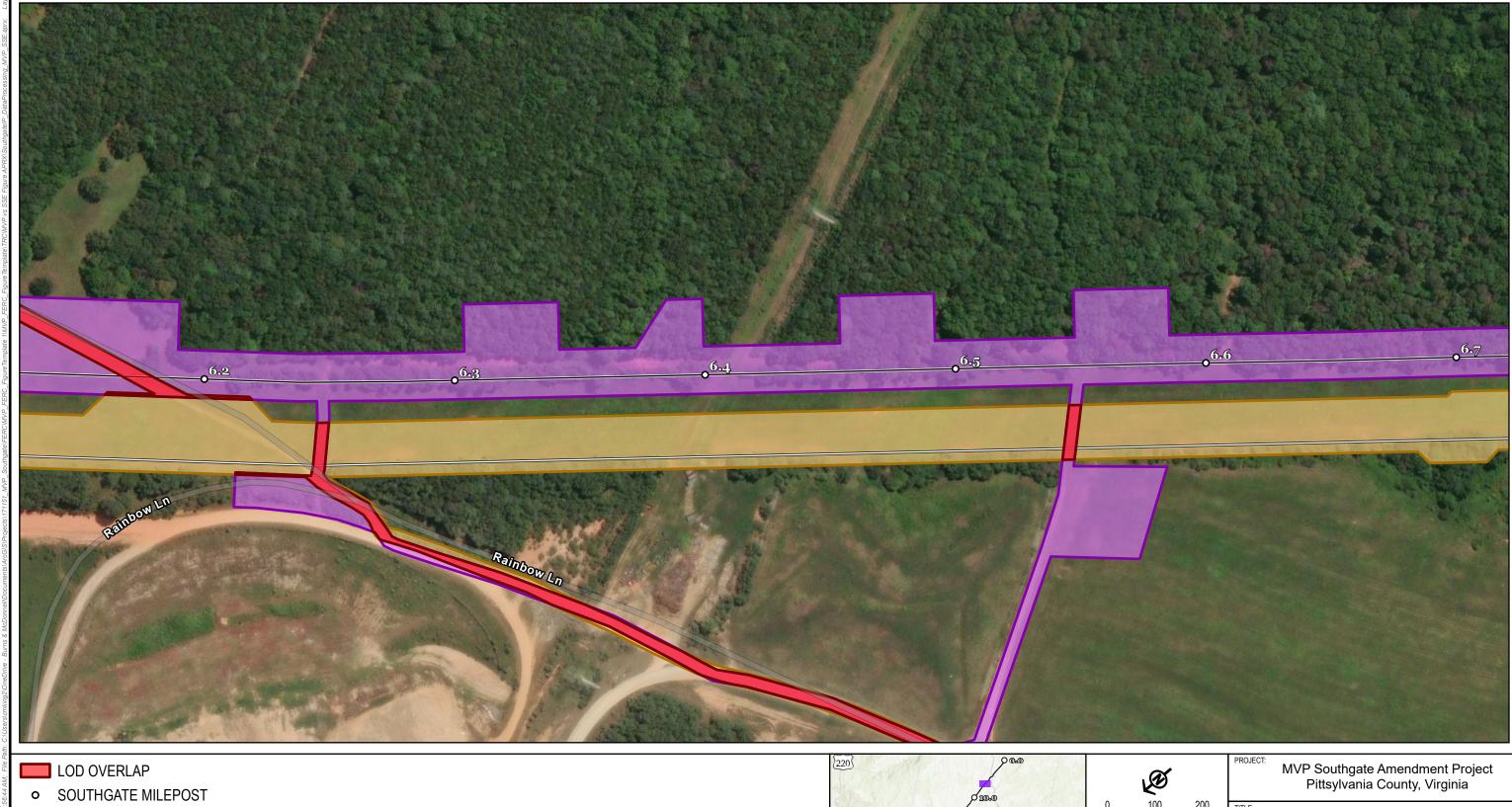
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

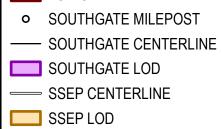
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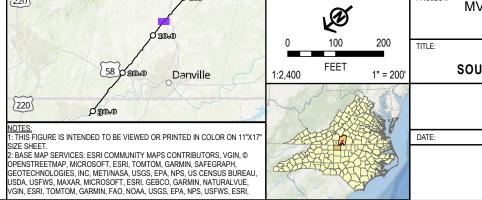
E: JULY 2025



Coordinate System: NAD 1927 BLM Zone 17N; Ma







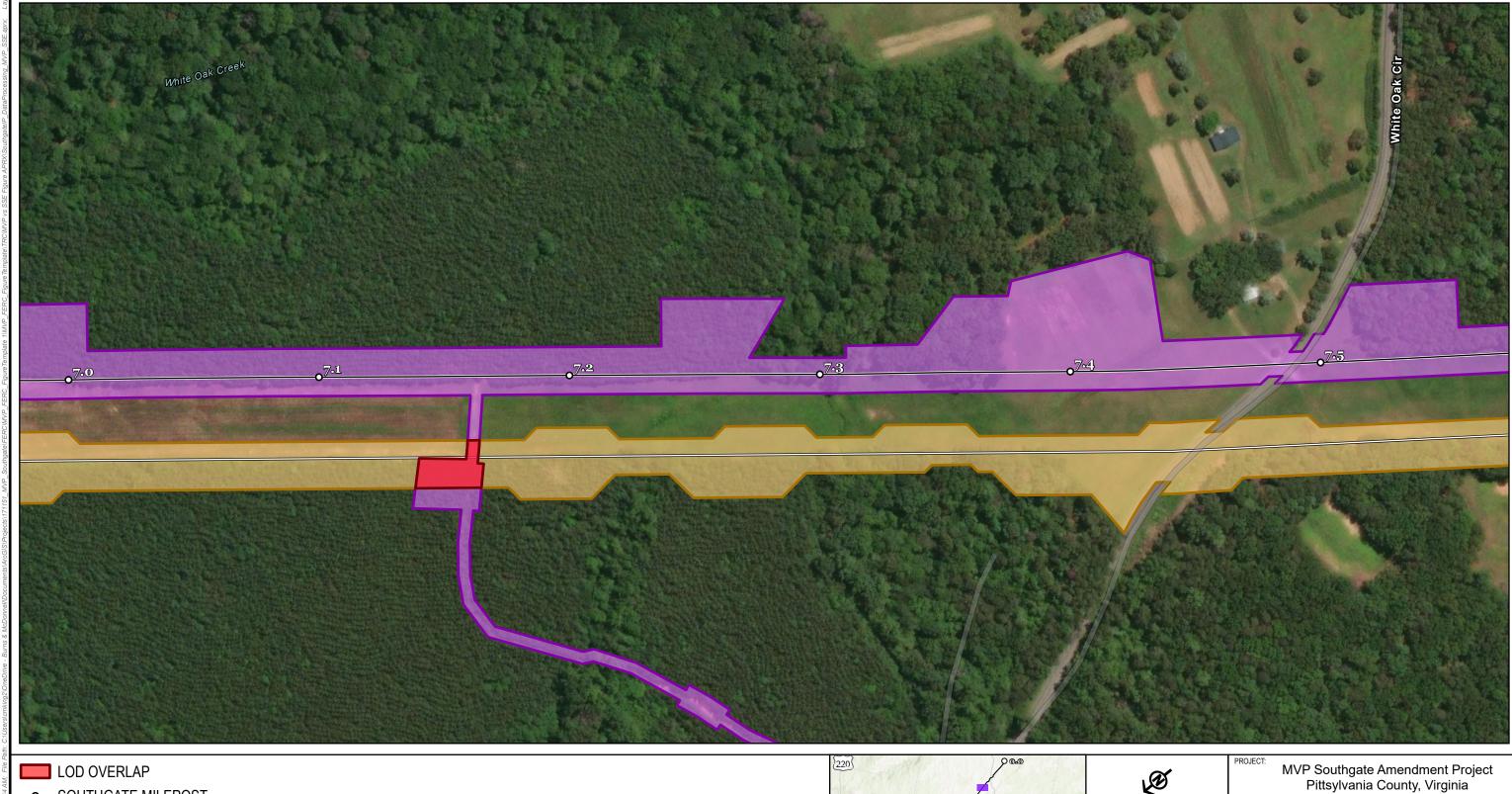
Pittsylvania County, Virginia

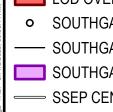
FIGURE 8C
SOUTHGATE LOD AND SSEP LOD

PAGE: 11
CL-015

DATE: JULY 2025







SOUTHGATE MILEPOST

SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD



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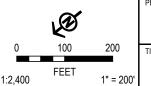
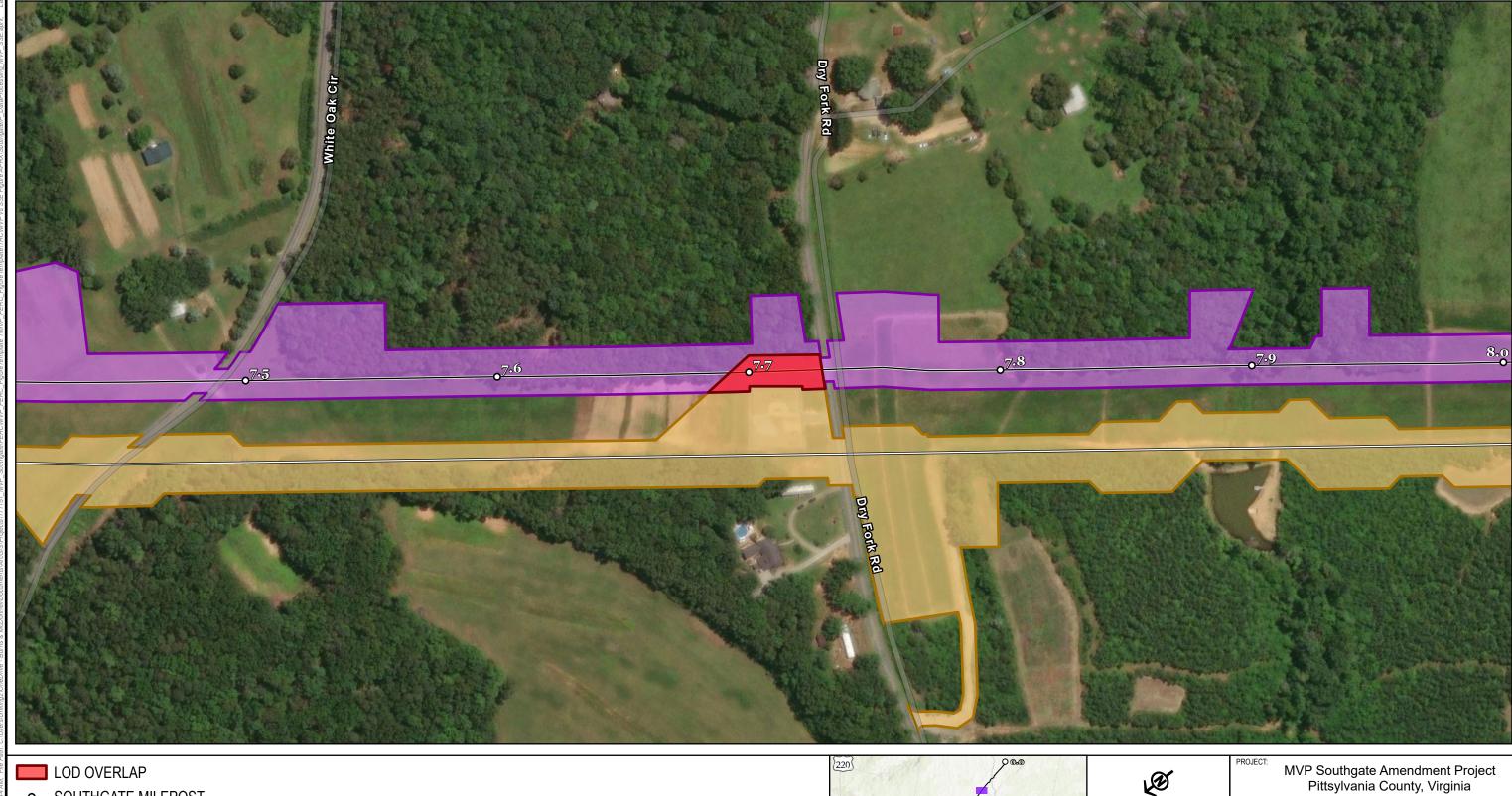


FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 12 CL-017







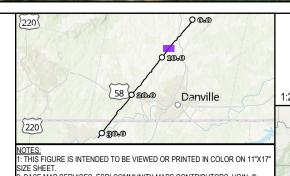
SOUTHGATE MILEPOST

- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD



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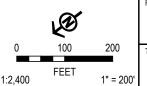
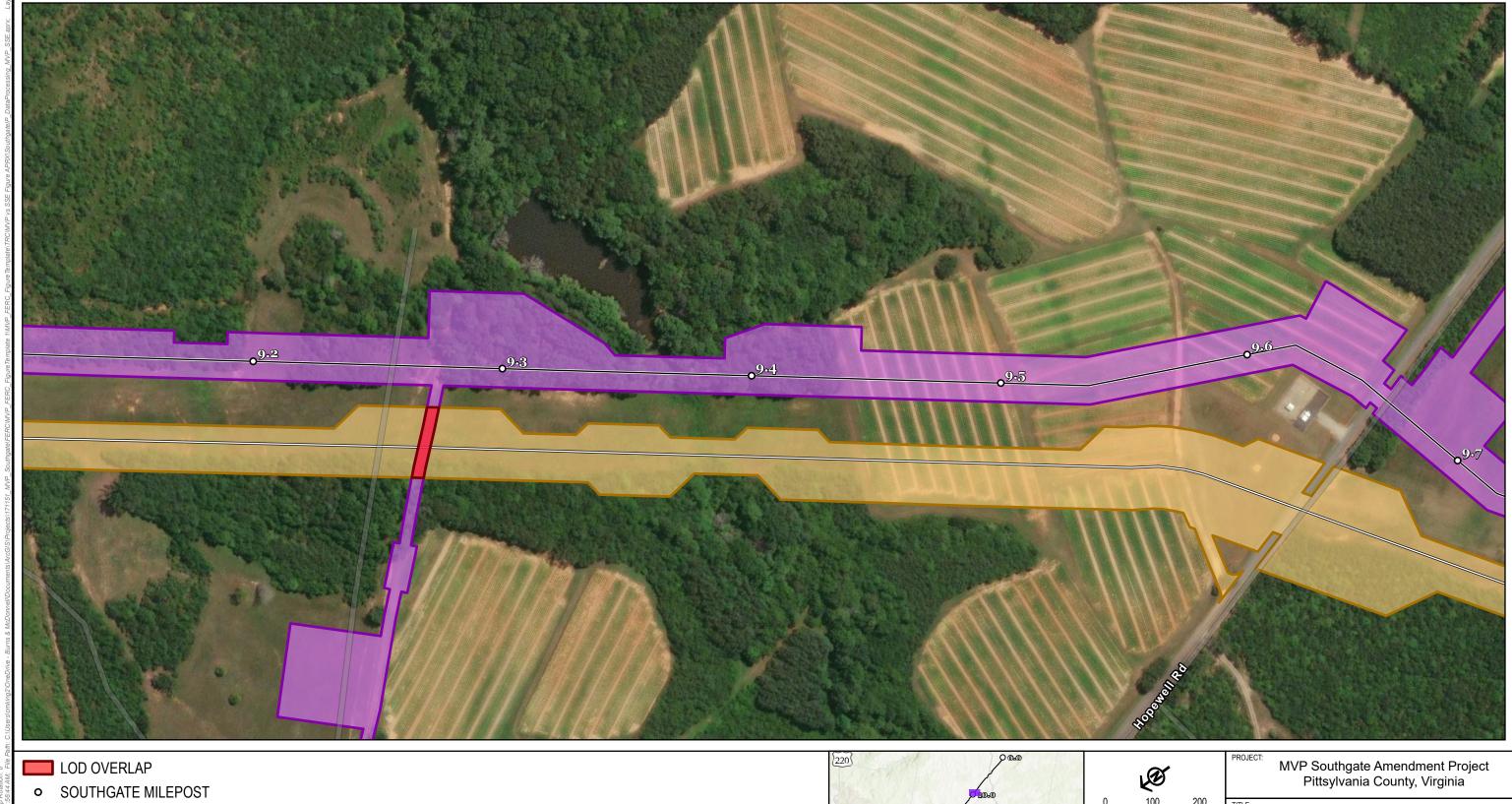
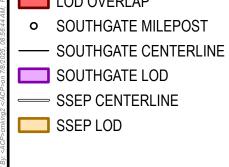


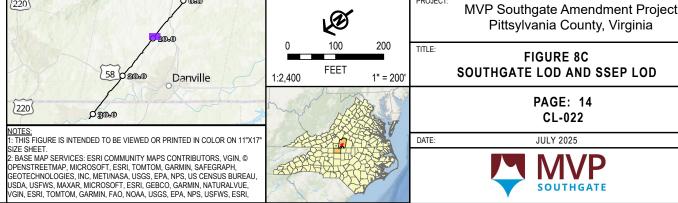
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

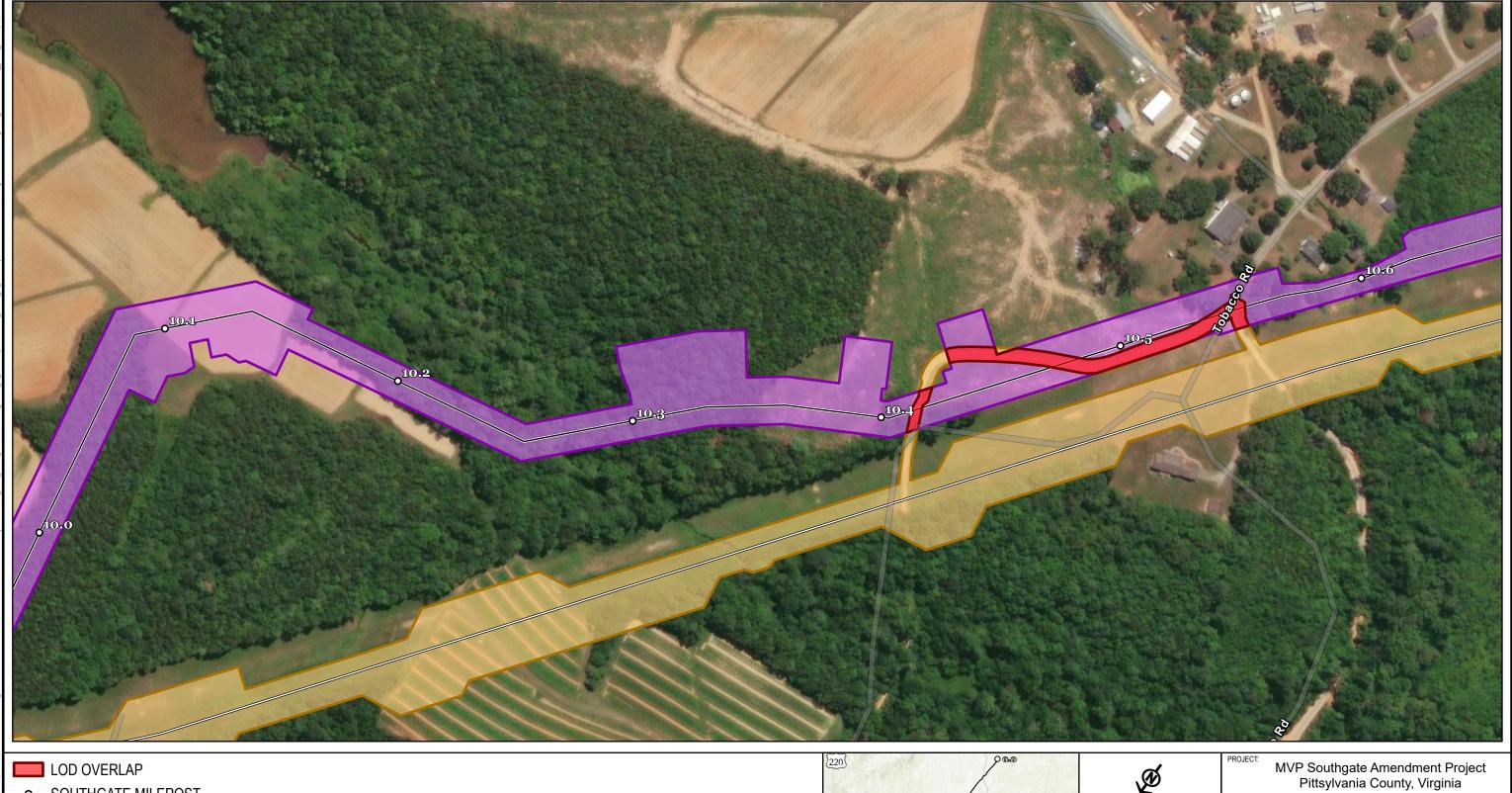
> PAGE: 13 CL-018



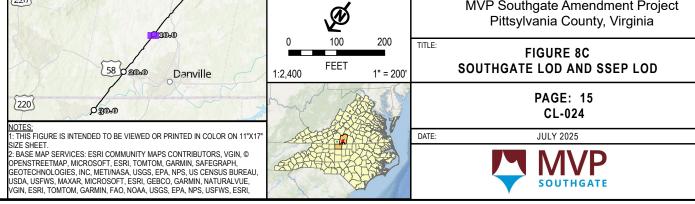


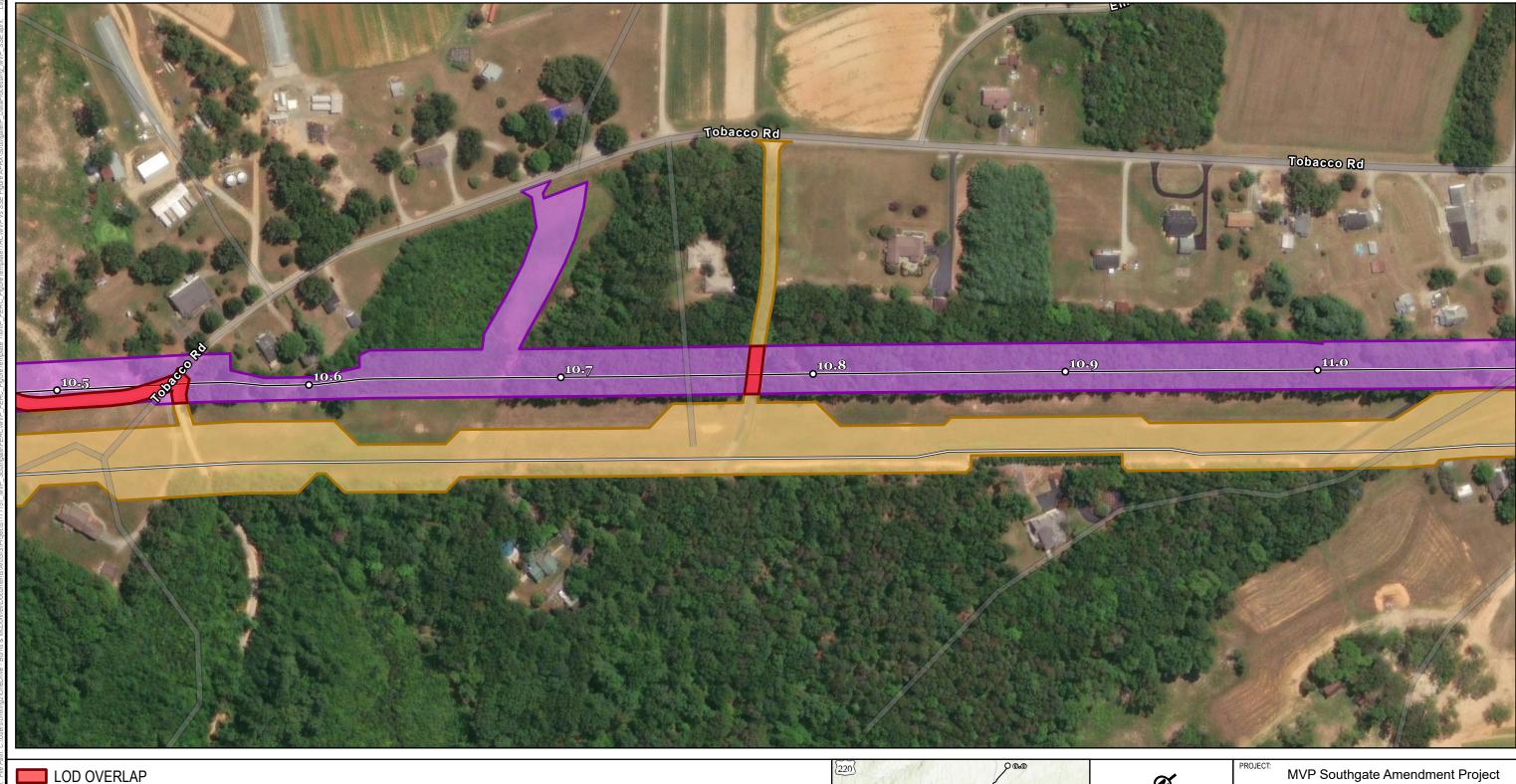












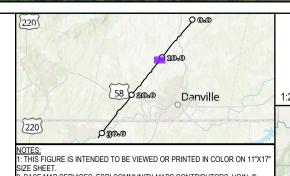


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

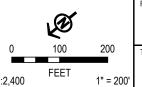
SSEP LOD



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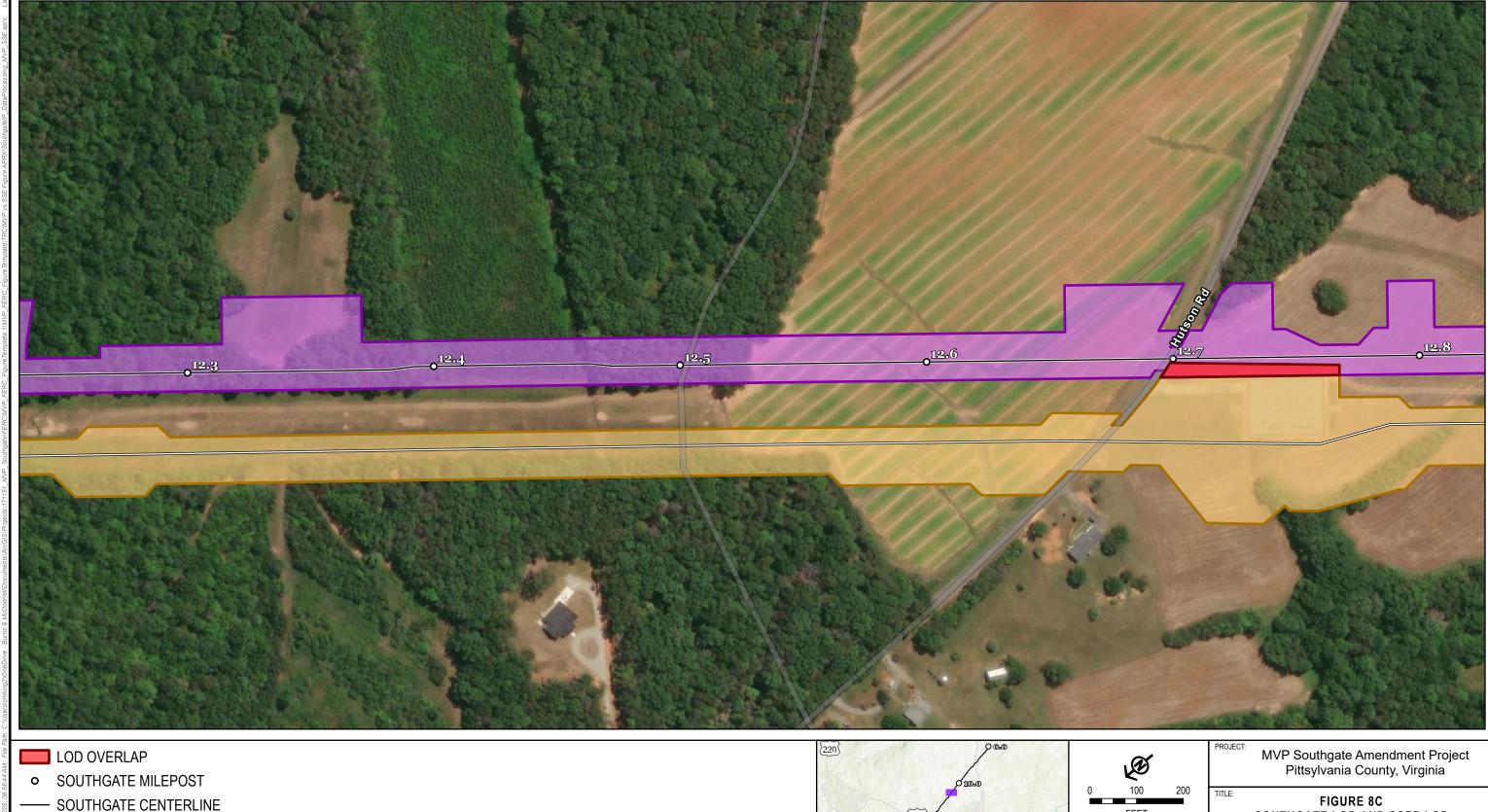


MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 16 CL-025





FEET

Danville

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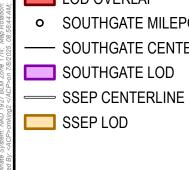
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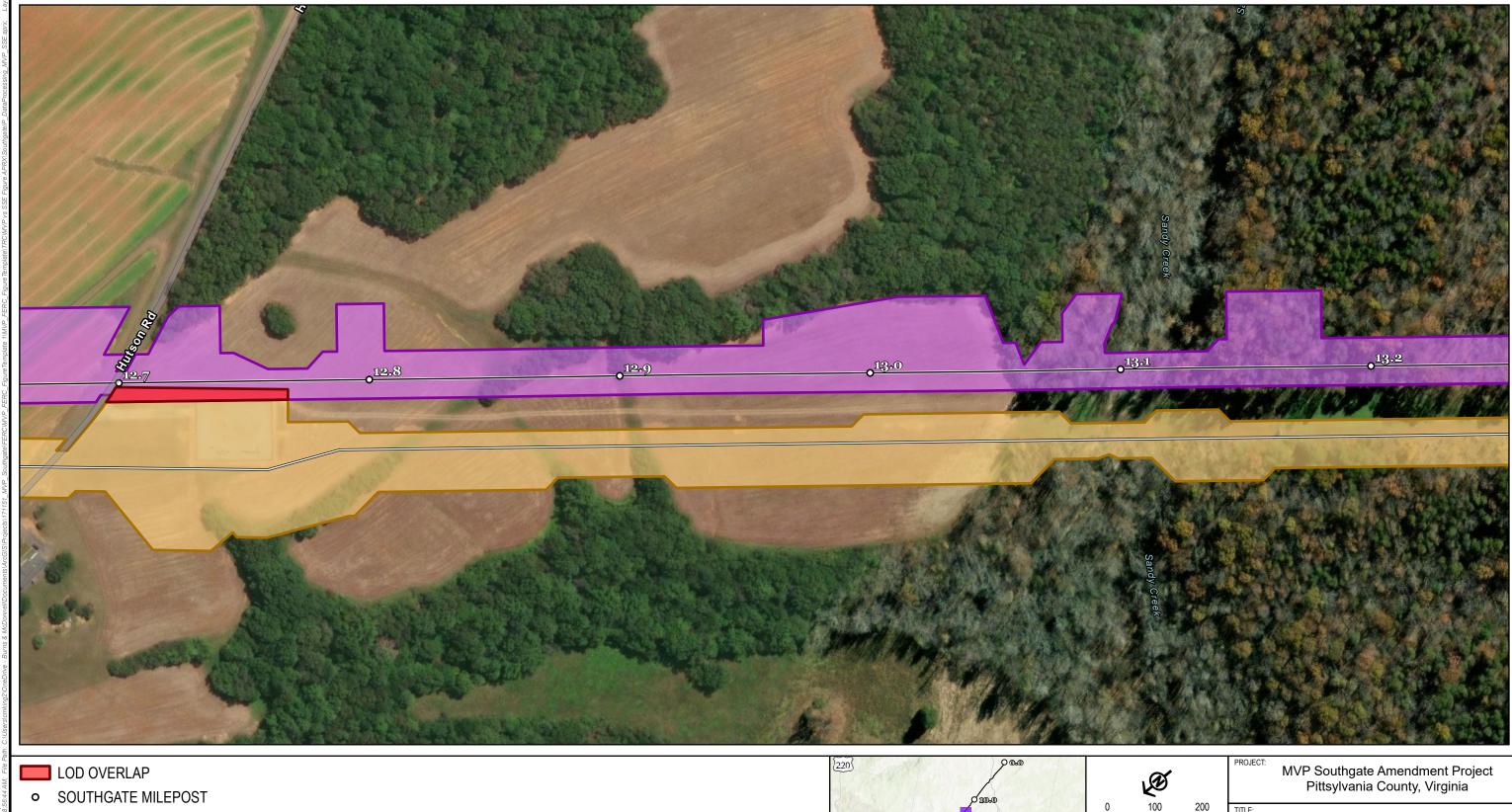
1" = 200'

SOUTHGATE LOD AND SSEP LOD

PAGE: 17

CL-029







- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD





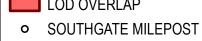
FEET

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 18 CL-030





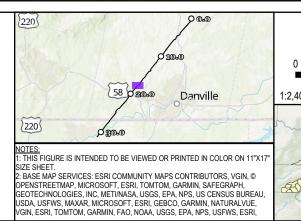


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD





FEET

200

1" = 200'

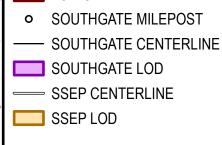
MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 19 CL-041







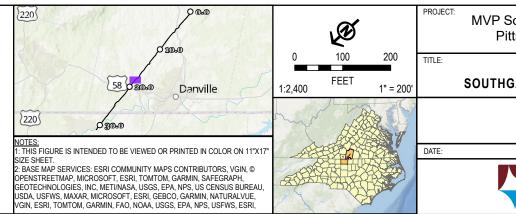


FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 20 CL-042







- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD

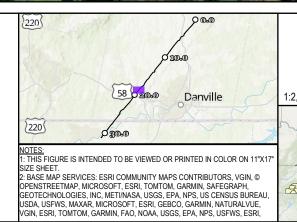




FIGURE 8C SOUTHGATE LOD AND SSEP LOD

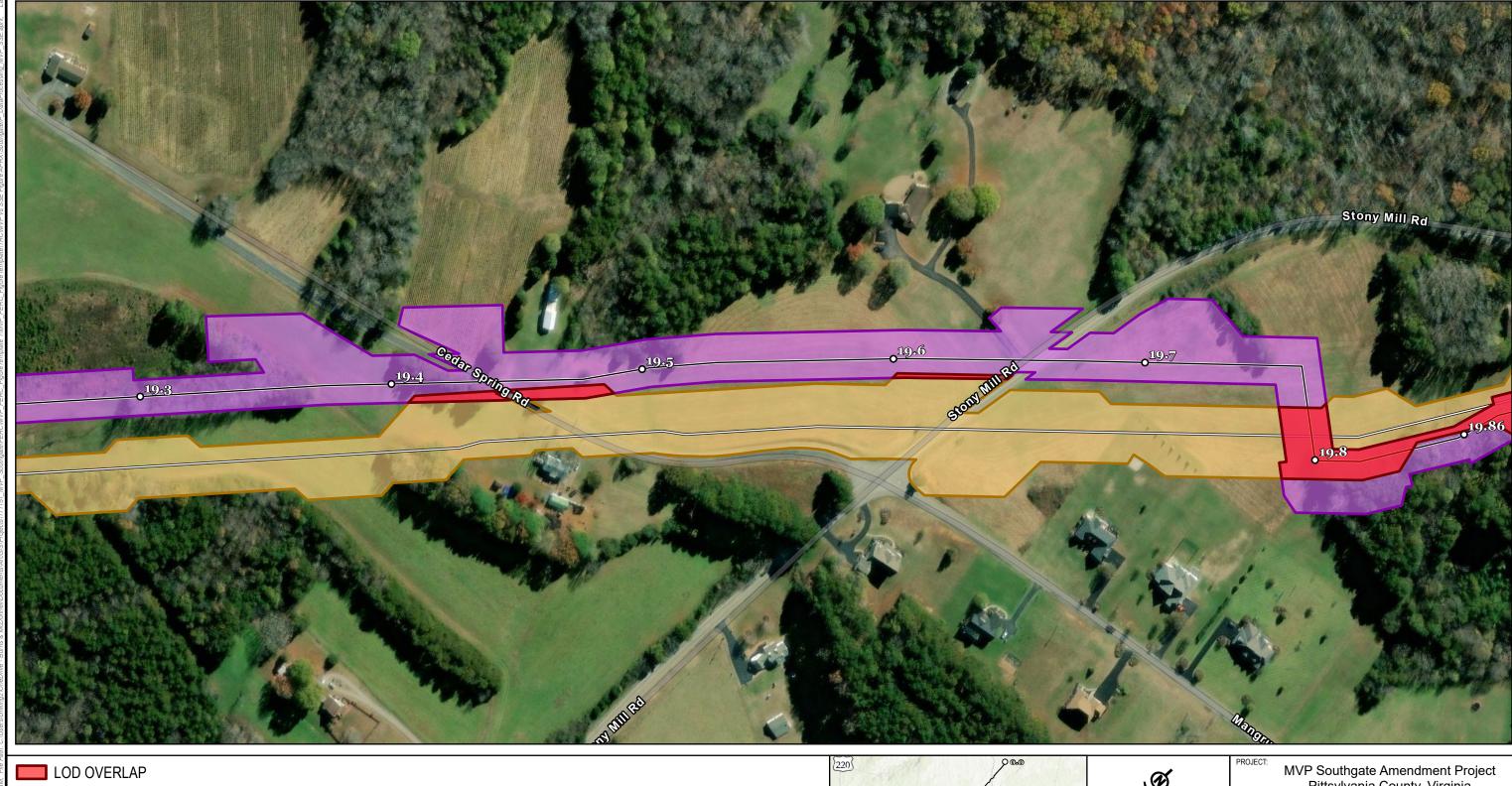
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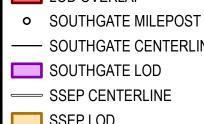
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200

1" = 200'







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VGIN, ESRI, TOMTOM, GARMIN, FAO, NOAA, USGS, EPA, NPS, USFWS, ESRI,

1" = 200' Danville

200

MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 22 CL-045

JULY 2025



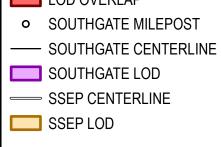
- SOUTHGATE CENTERLINE

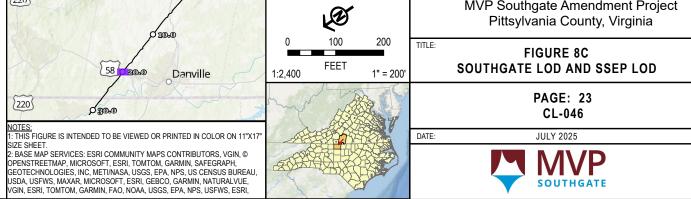
SOUTHGATE LOD

— SSEP CENTERLINE

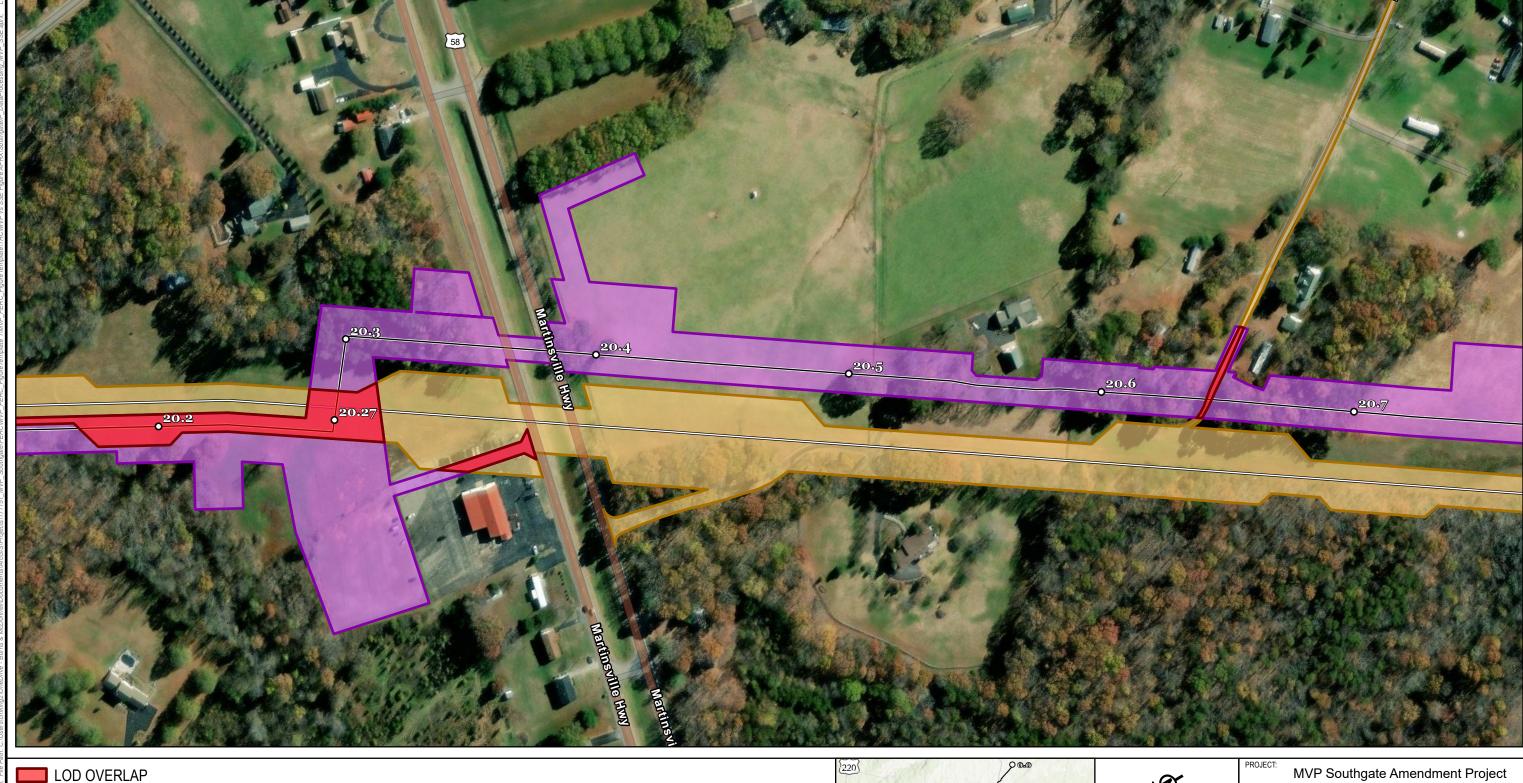
SSEP LOD

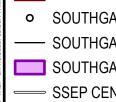






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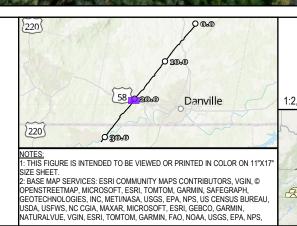


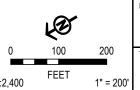


 SOUTHGATE MILEPOST - SOUTHGATE CENTERLINE SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD



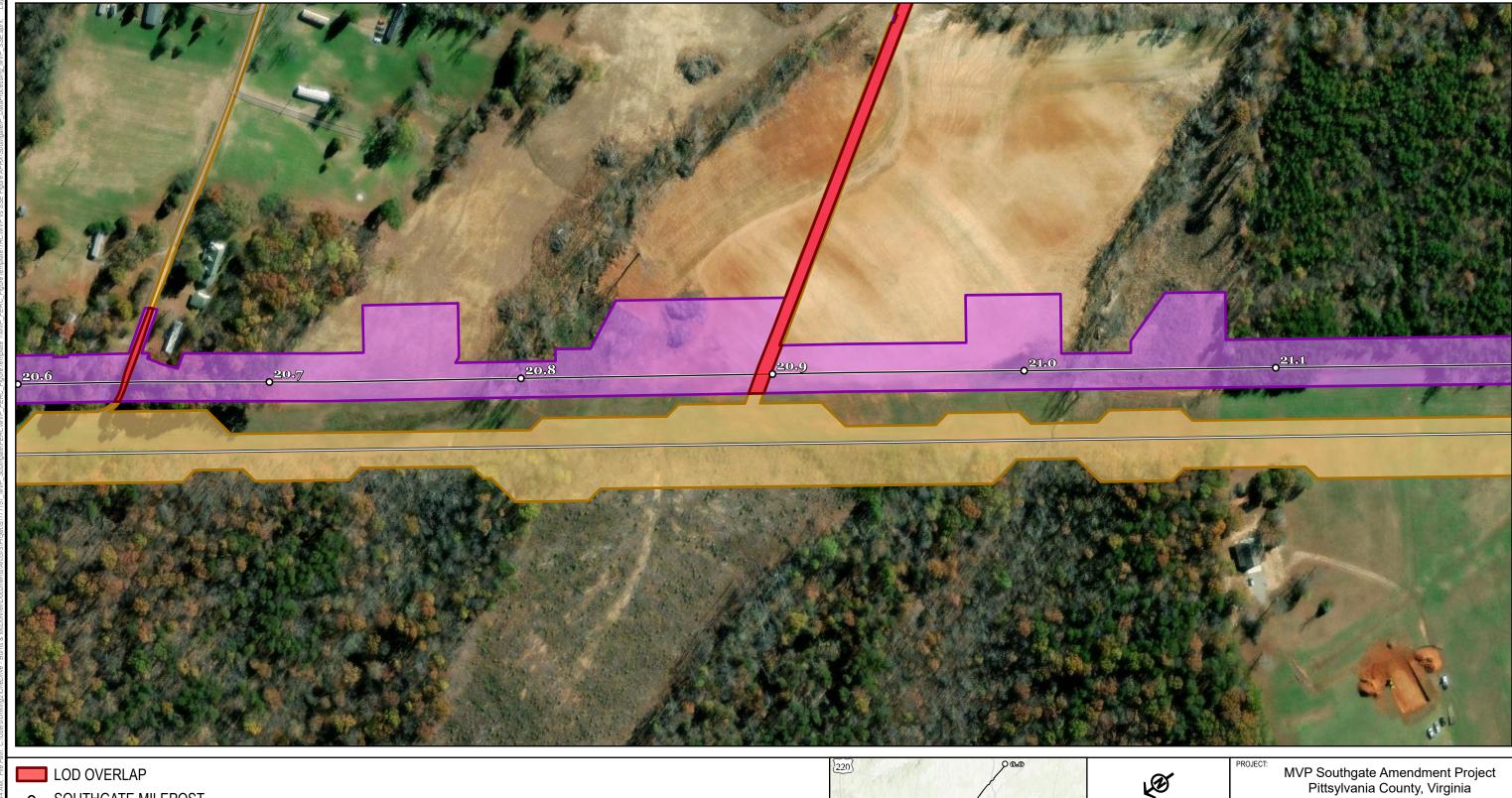


MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 24 CL-047





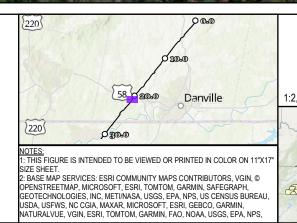


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

—— SSEP CENTERLINE

SSEP LOD





200

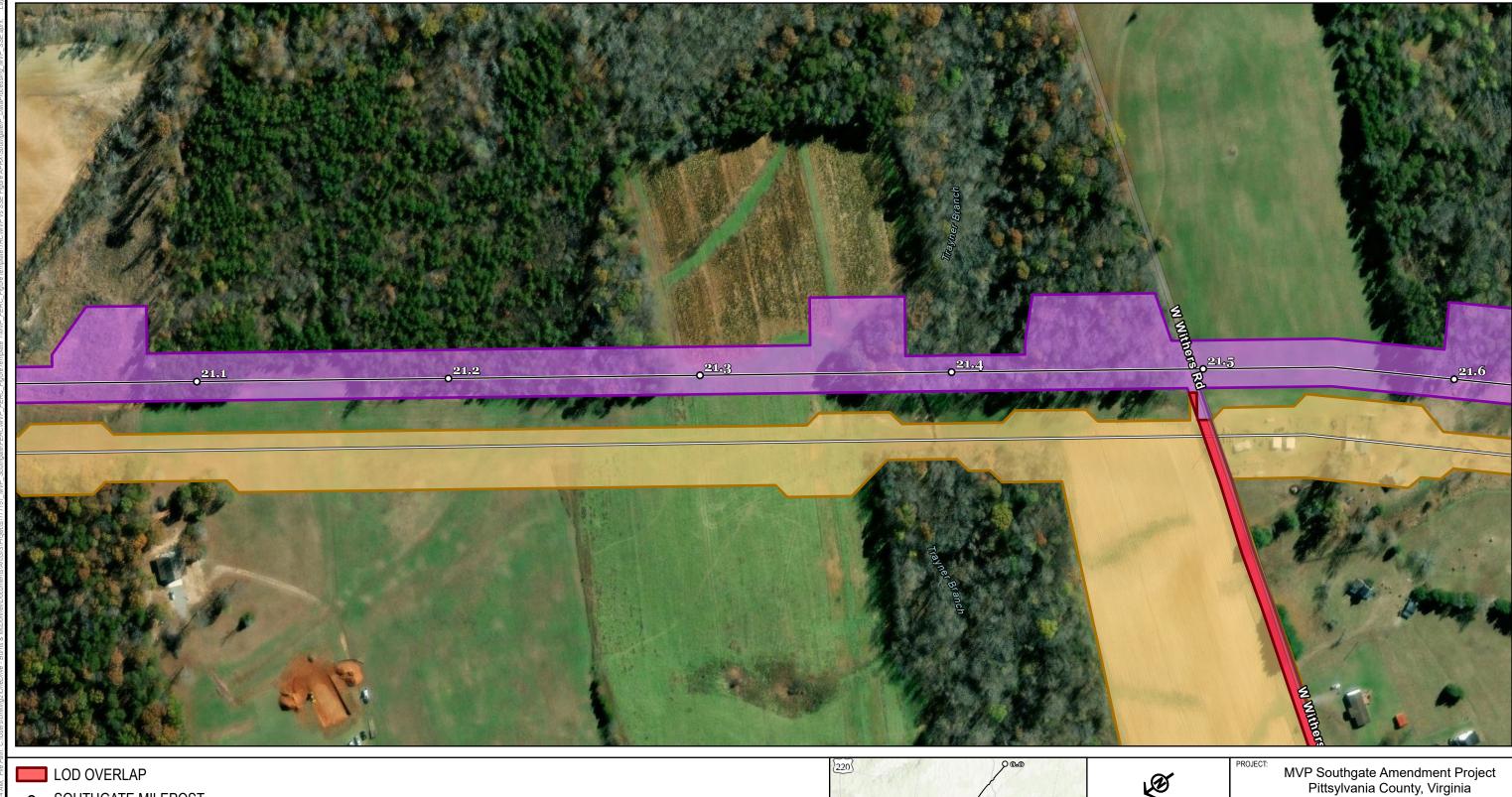
1" = 200'

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 25 CL-048

: JULY 2025







SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD

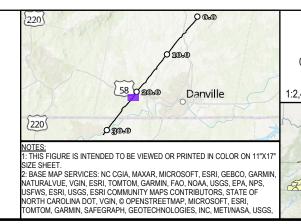


FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 26 CL-049

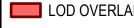
JULY 2025

200

1" = 200'





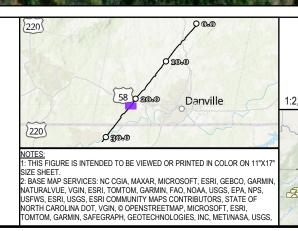


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD





200

1" = 200'

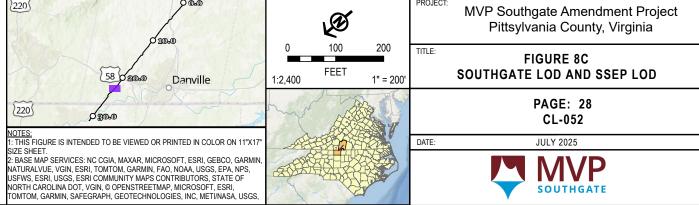
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

PAGE: 27 CL-050













- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD





FEET

1" = 200'

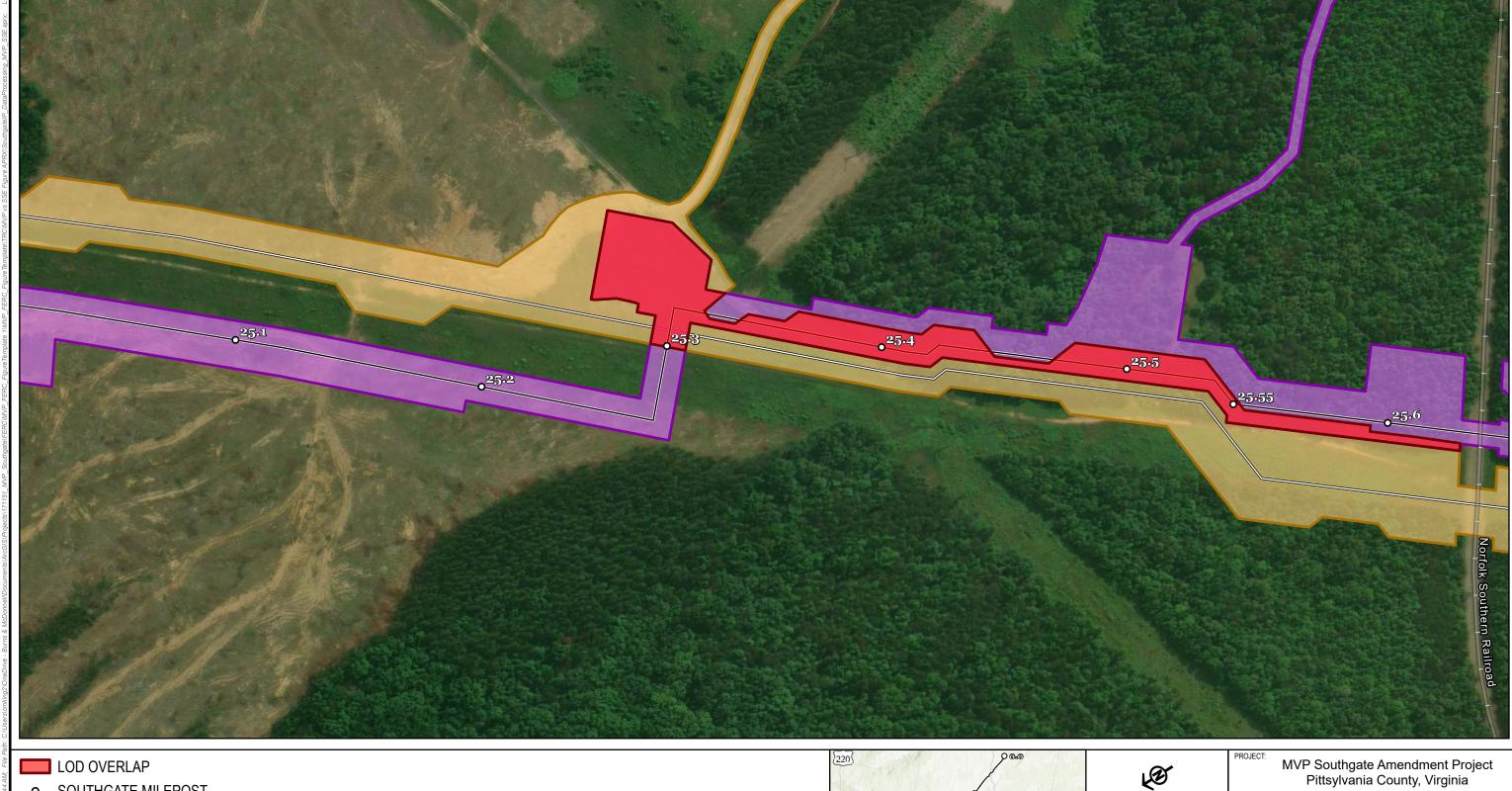
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FIGURE 8C SOUTHGATE LOD AND SSEP LOD

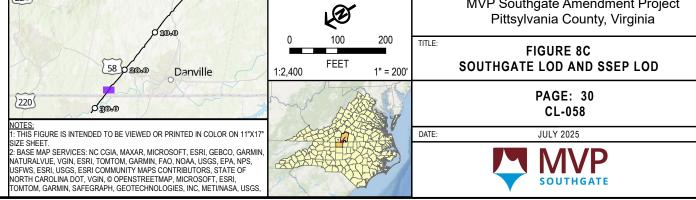
> PAGE: 29 CL-053

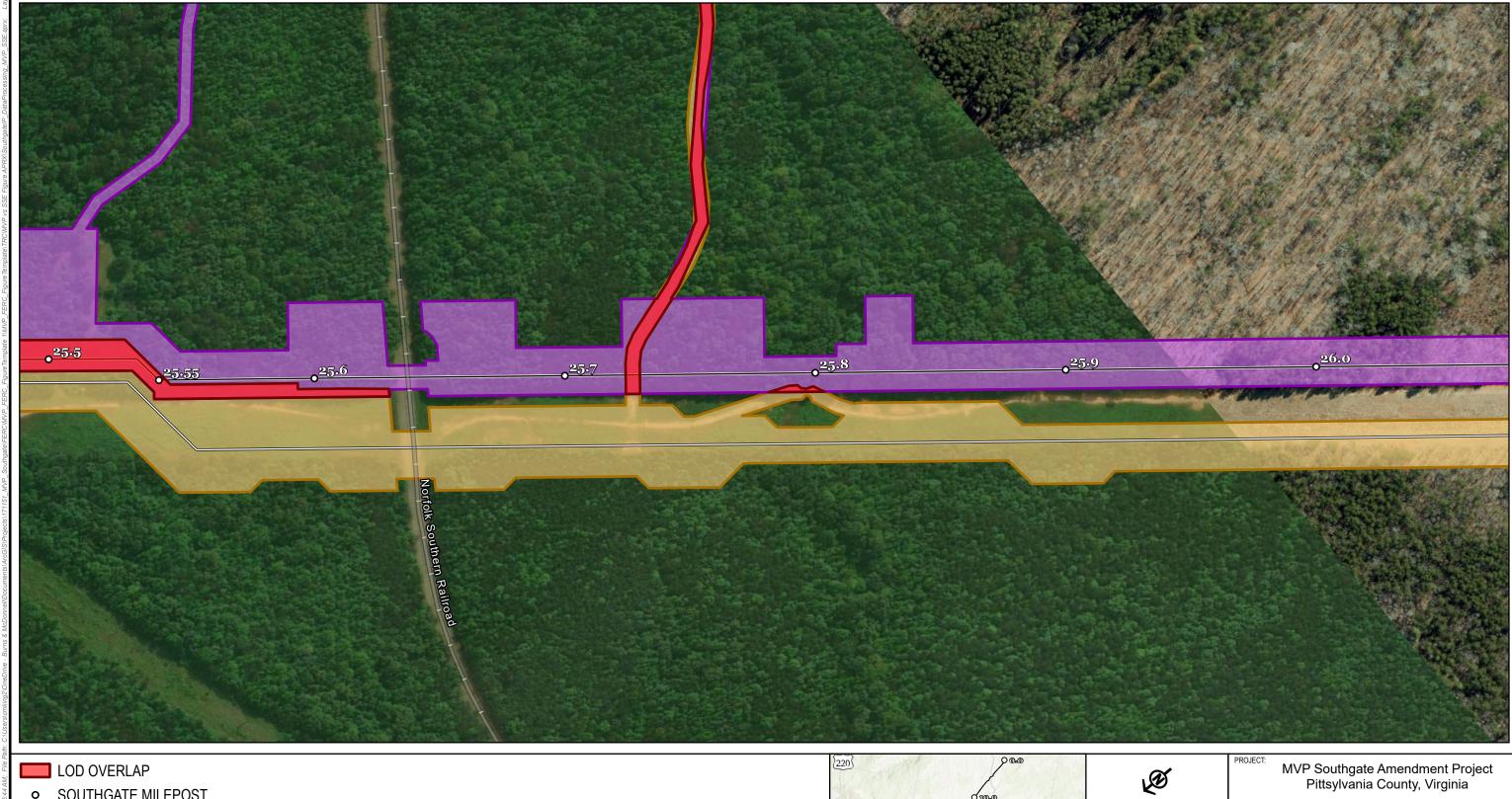
JULY 2025

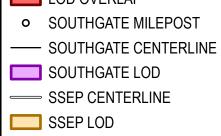
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2: BASE MAP SERVICES: NC CGIA, MAXAR, MICROSOFT, ESRI, GEBCO, GARMIN,
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NORTH CAROLINA DOT, VGIN, © OPENSTREETMAP, MICROSOFT, ESRI,
TOMTOM, GARMIN, SAFEGRAPH, GEOTECHNOLOGIES, INC, METI/NASA, USGS,











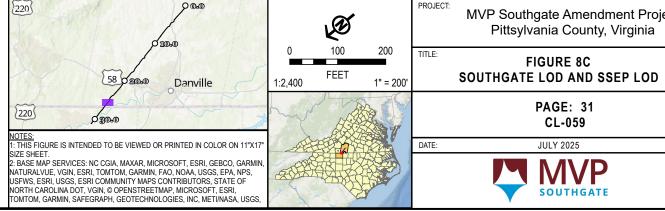
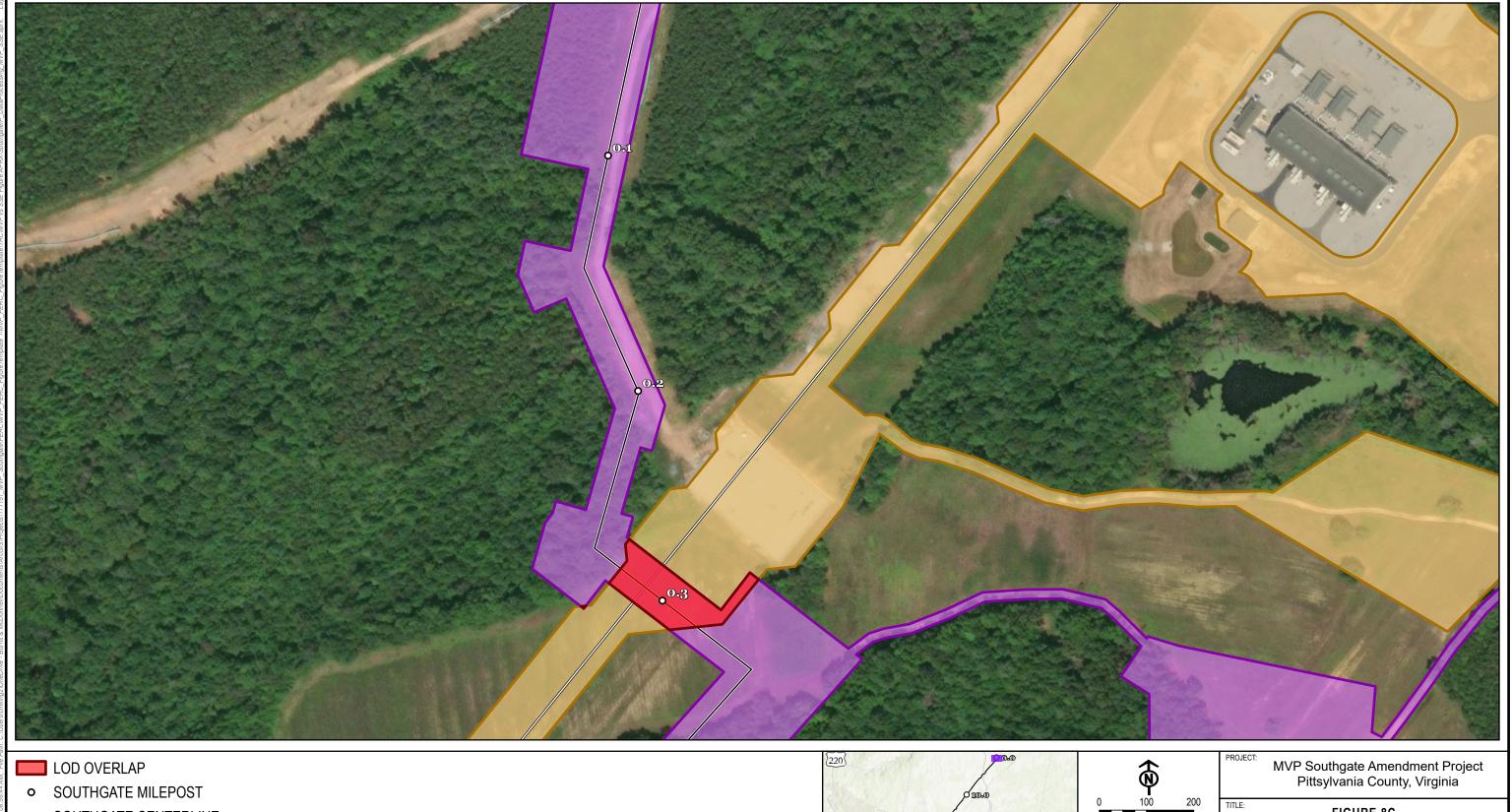
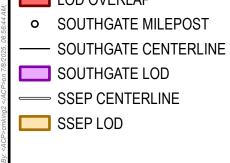


FIGURE 8C

PAGE: 31

CL-059





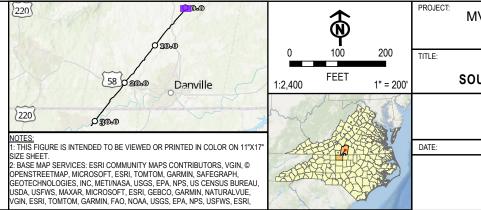
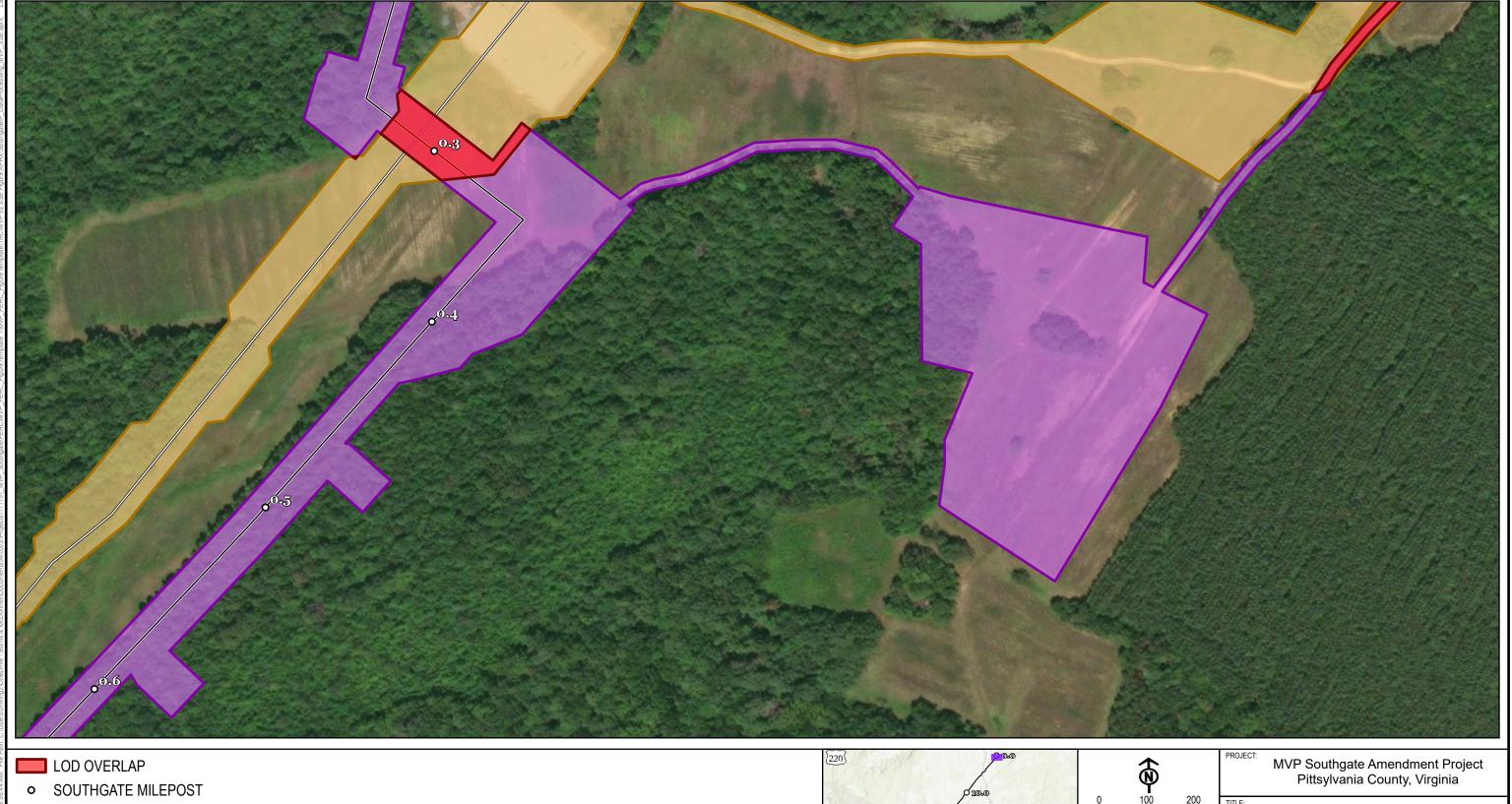


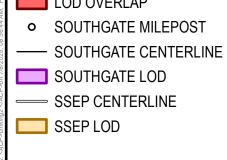
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

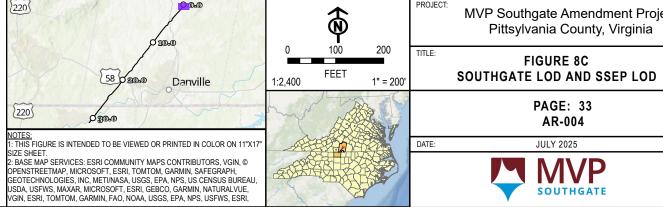
> PAGE: 32 AR-003

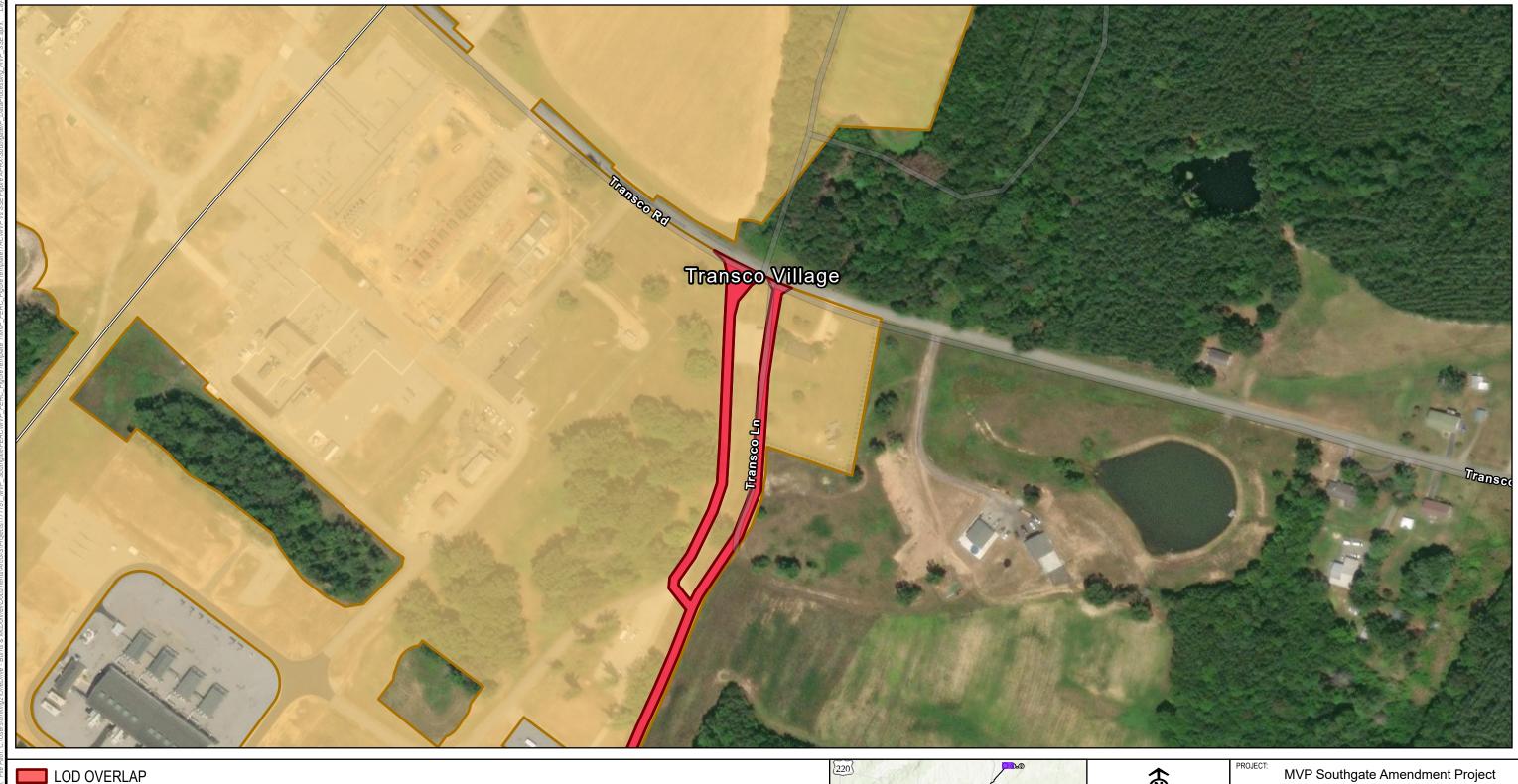
ATE: JULY 2025





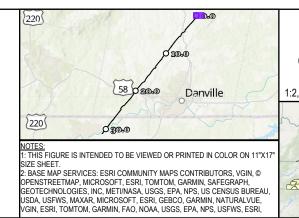


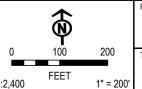






 SOUTHGATE MILEPOST - SOUTHGATE CENTERLINE SOUTHGATE LOD —— SSEP CENTERLINE SSEP LOD



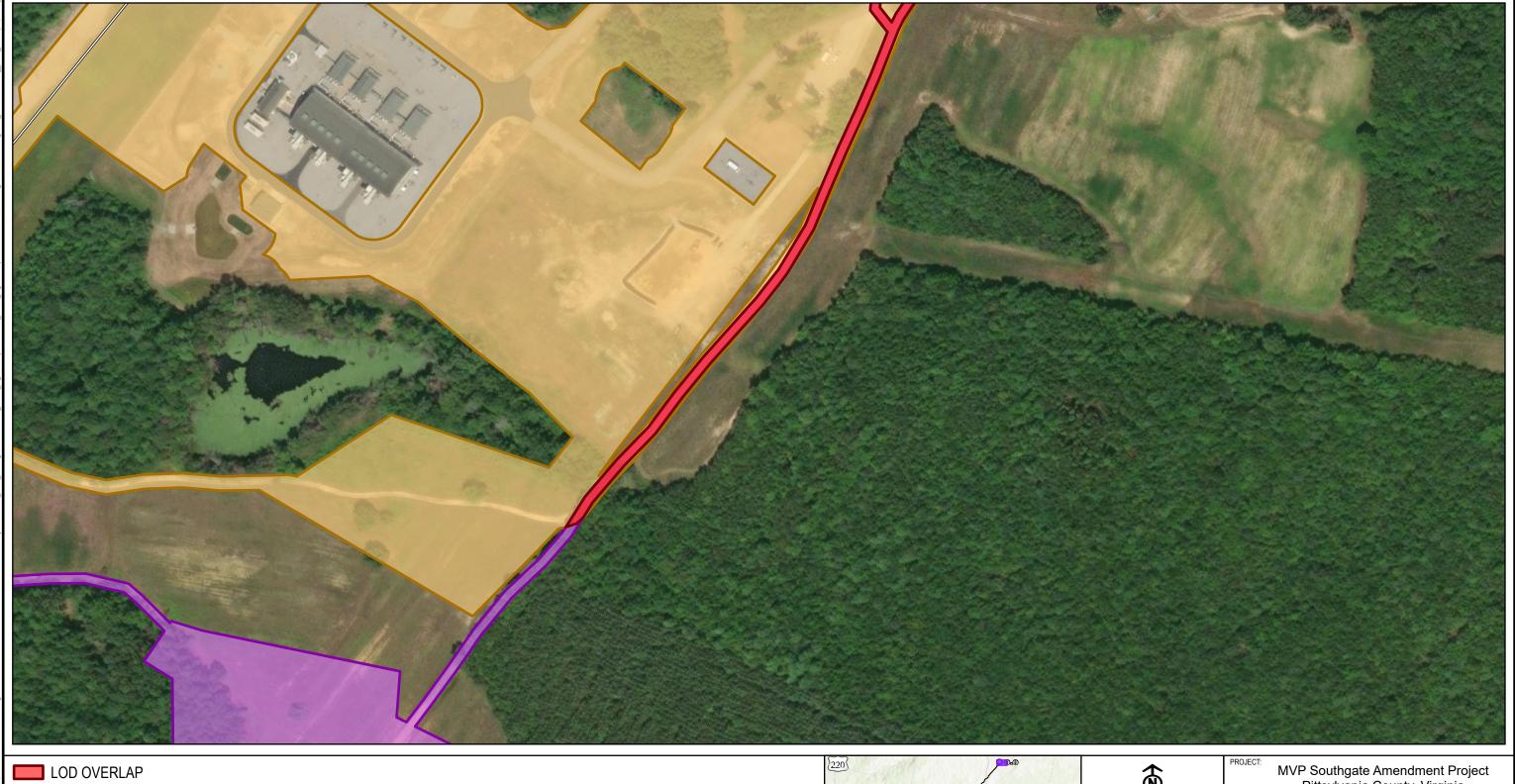


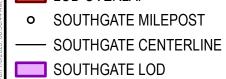
MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 34 AR-005

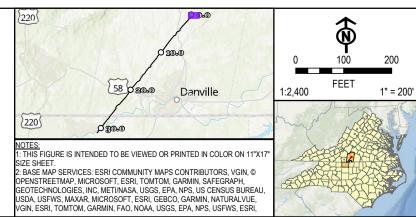






— SSEP CENTERLINE

SSEP LOD

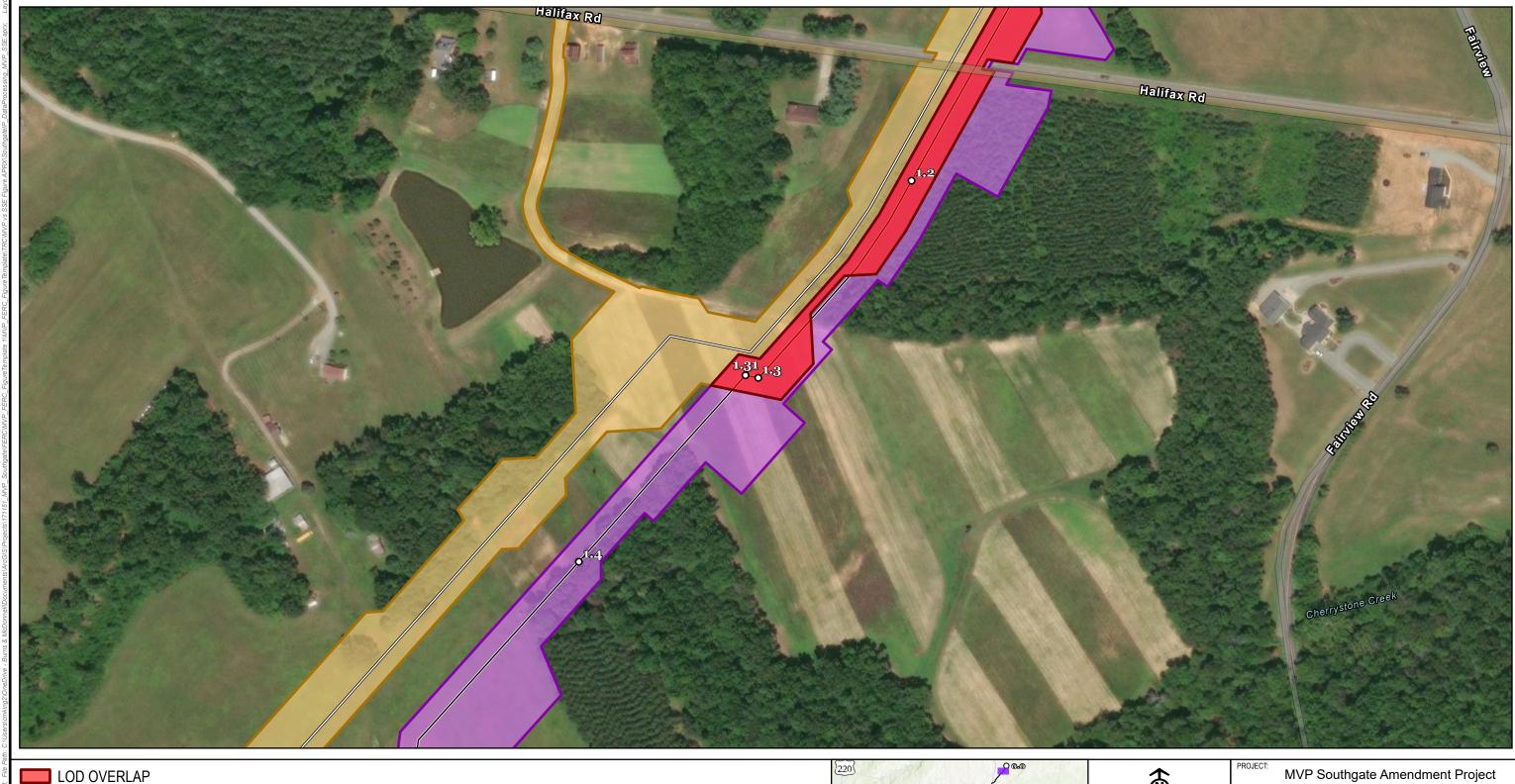


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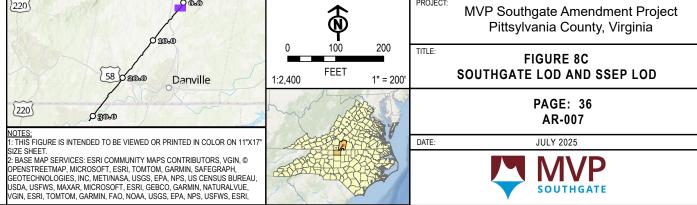
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 35 AR-006

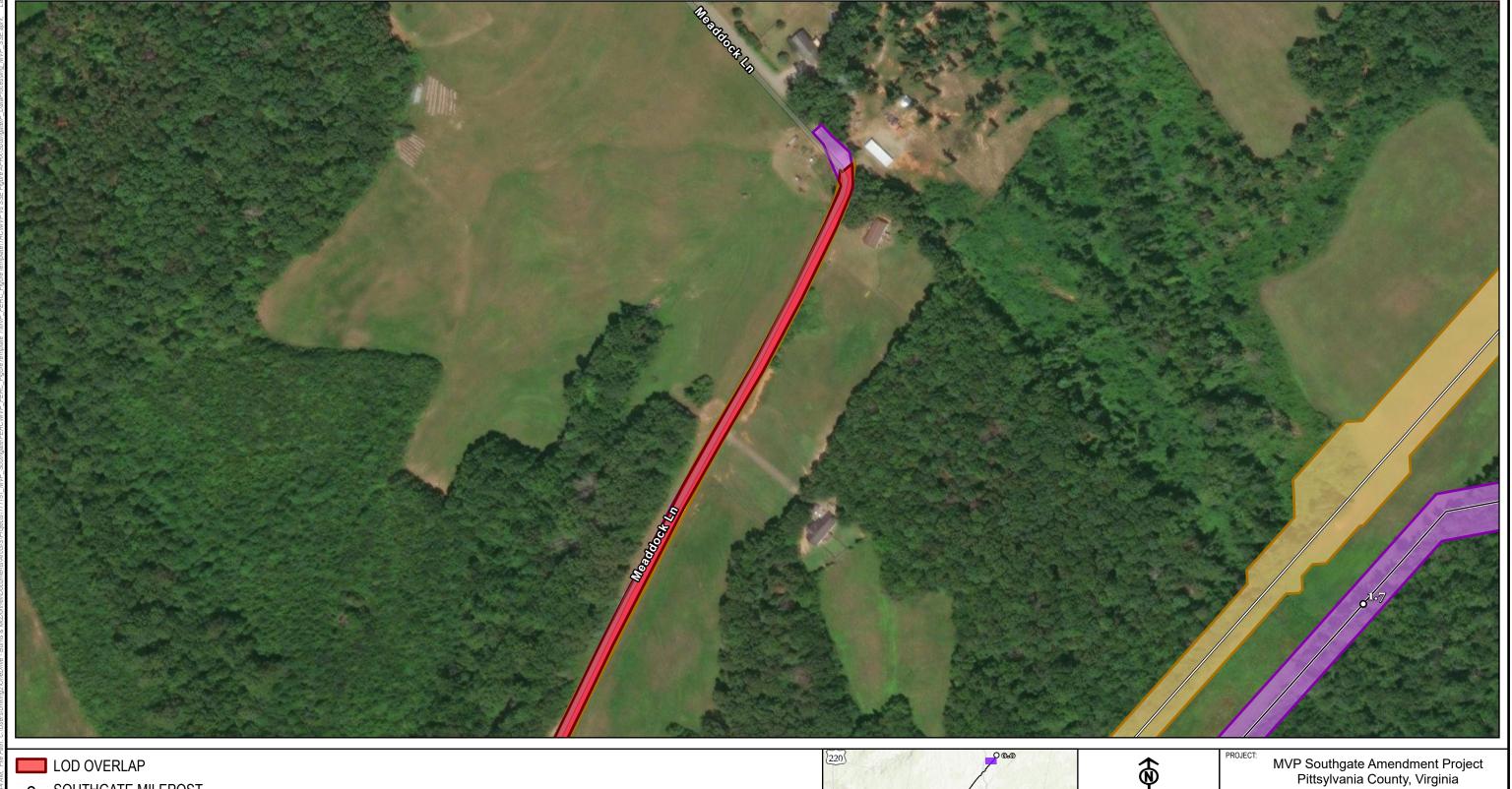




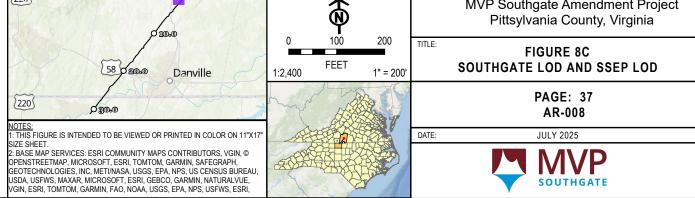




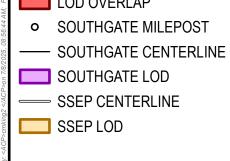
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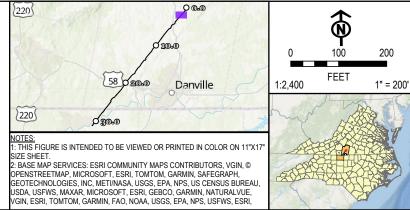
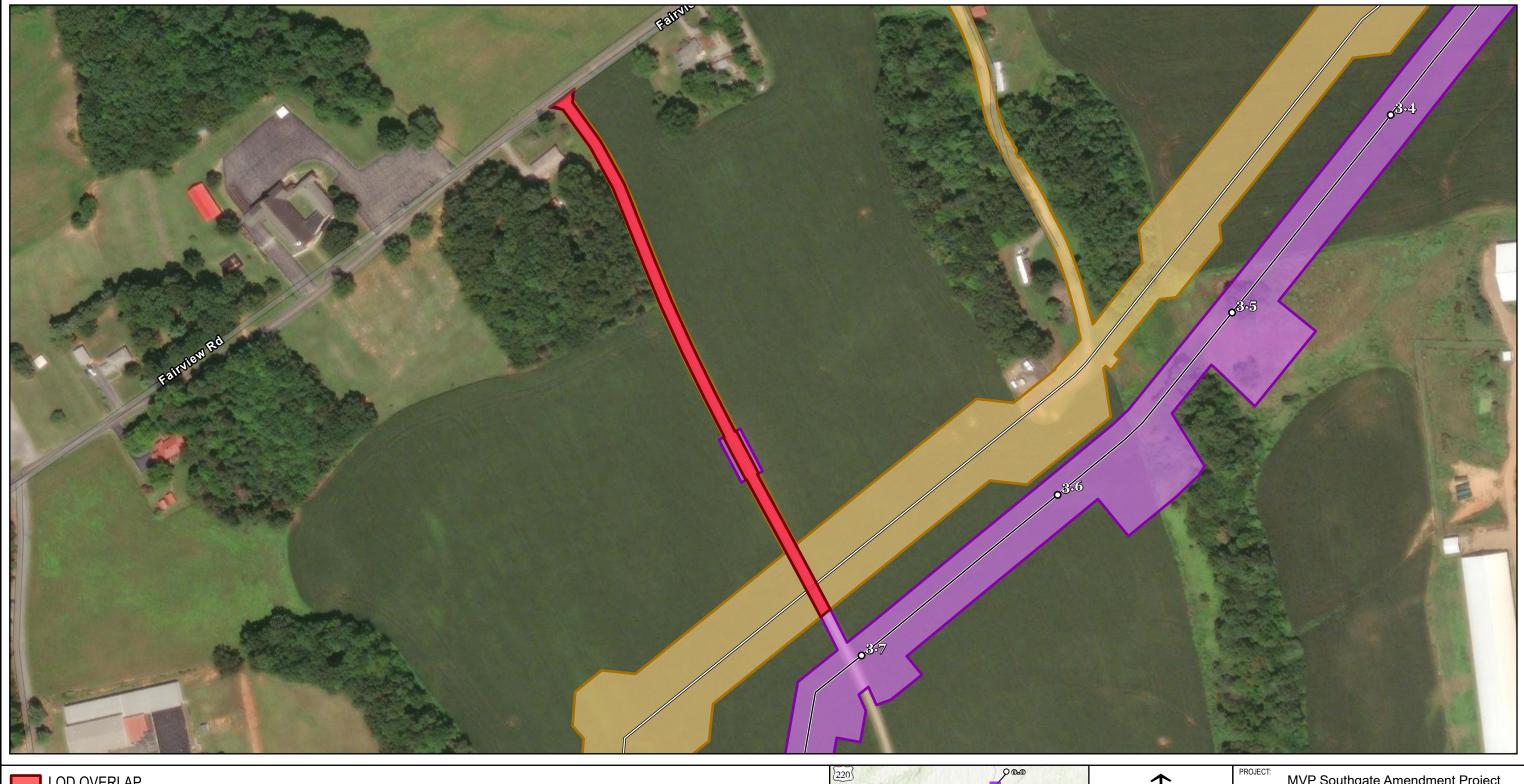


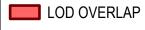
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 38 AR-009

ATE: JULY 2025





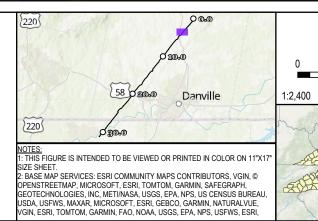


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

—— SSEP CENTERLINE

SSEP LOD



MVP Southgate Amendment Project
Pittsylvania County, Virginia

TITLE:

FIGURE 8C

FEET

1" = 200'

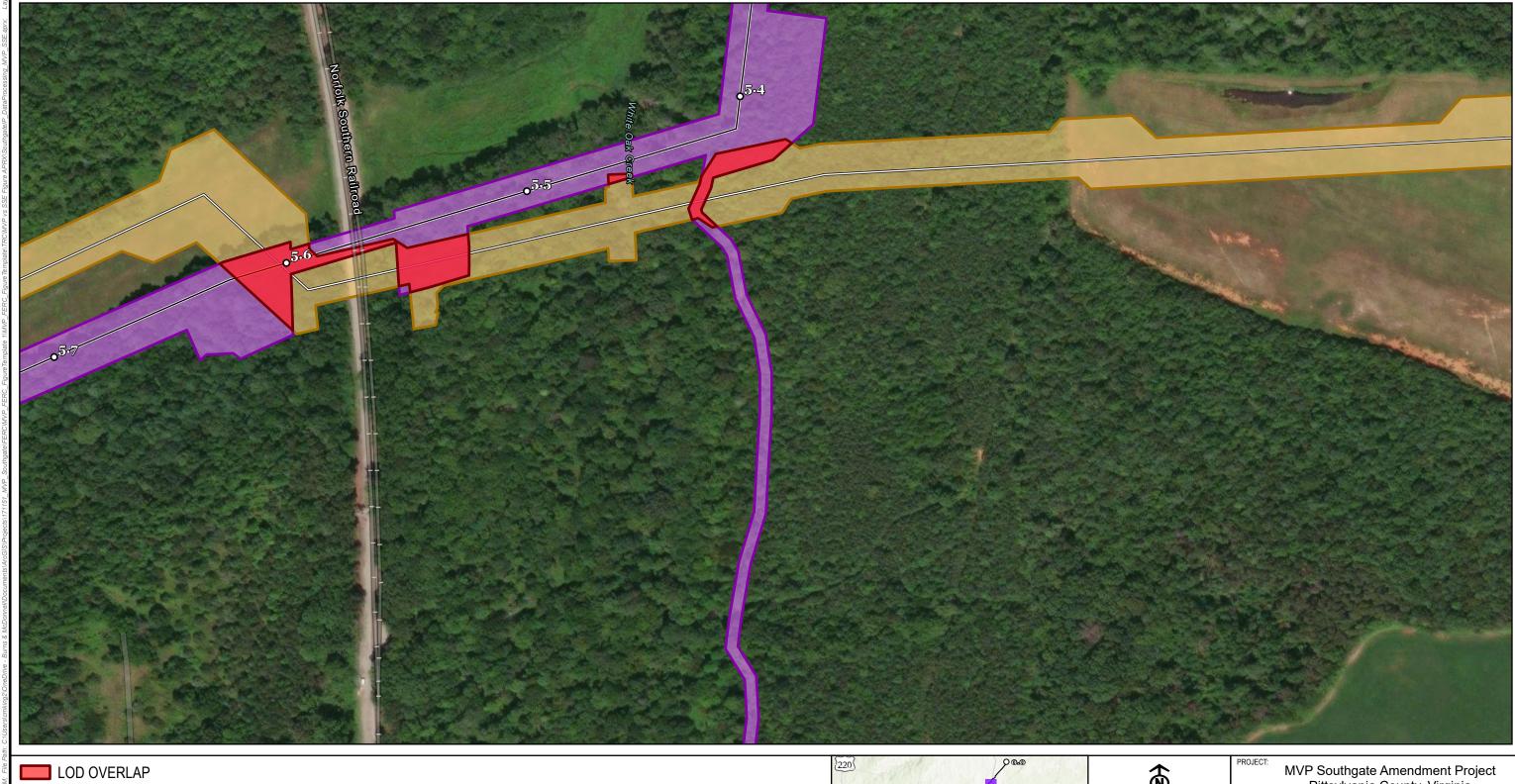
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 39 AR-013

ATE: JULY 2025



But System: NAD 1927 BLM Zone 17N; Map Rotation

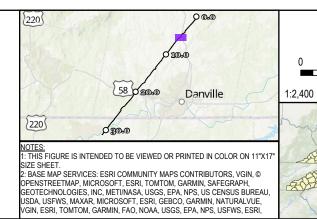


SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD



FEET

200

1" = 200'

MVP Southgate Amendment Project Pittsylvania County, Virginia

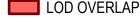
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 40 AR-018







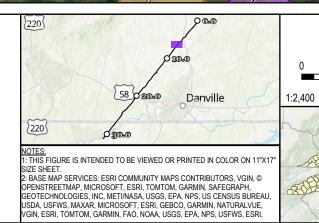


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

—— SSEP CENTERLINE

SSEP LOD



200 FEET 1" = 200'

MVP Southgate Amendment Project Pittsylvania County, Virginia

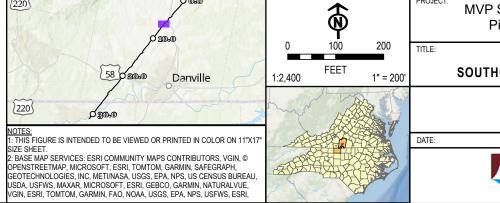
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 41 AR-019







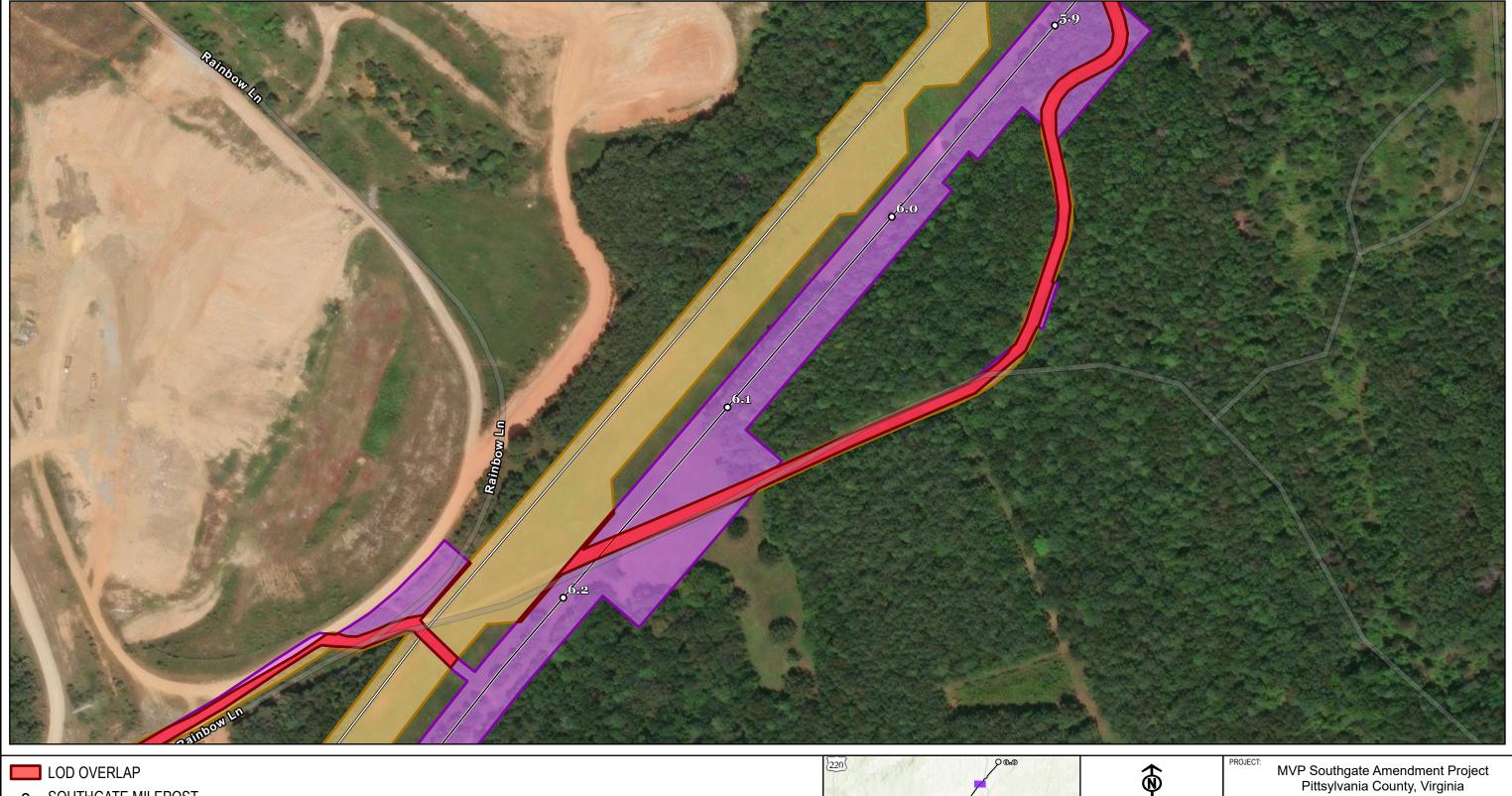


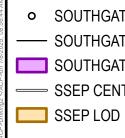
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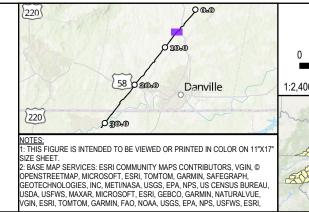
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 42 AR-020









200 FEET 1" = 200' 1:2,400

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 43 AR-021



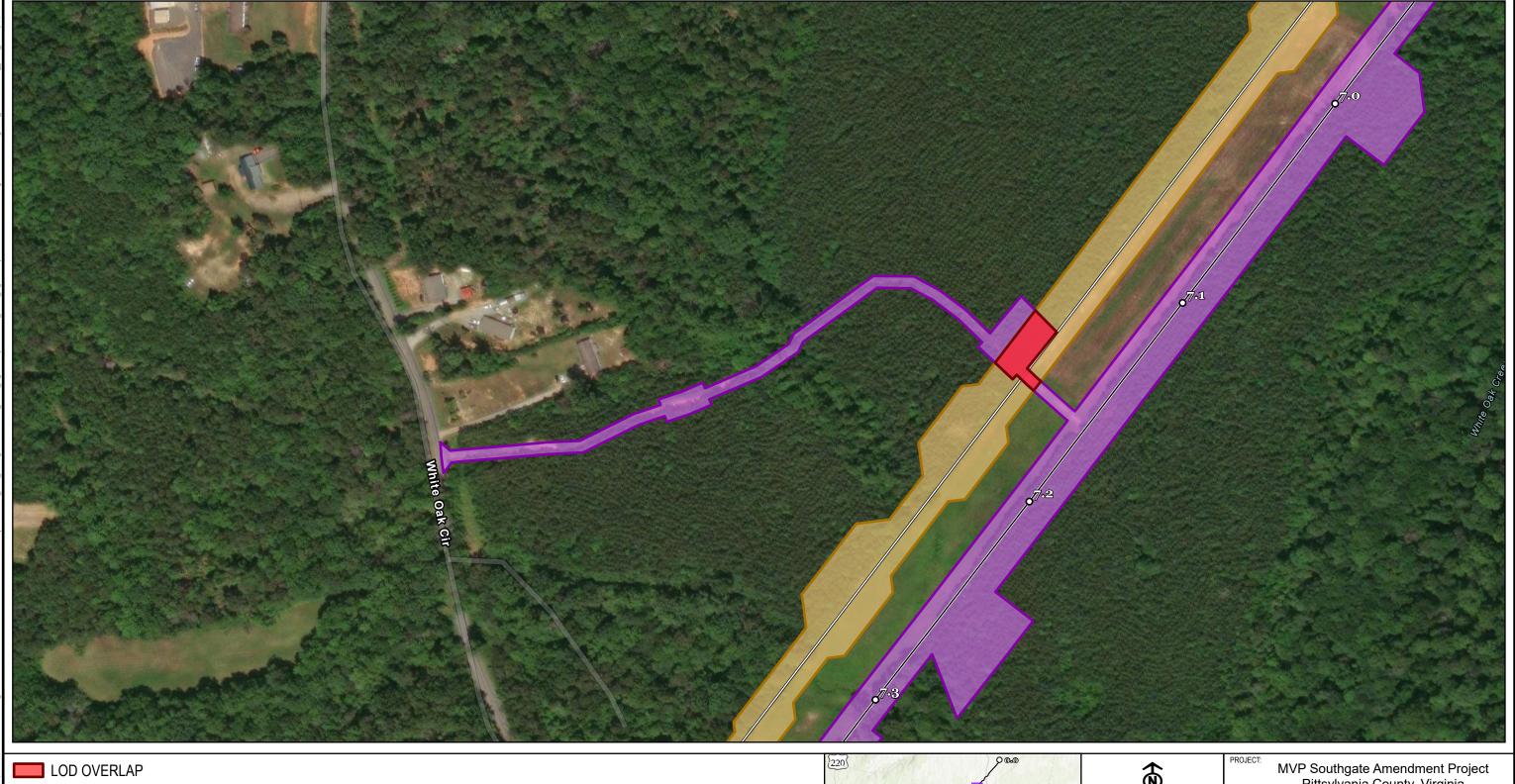
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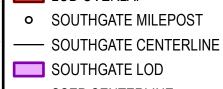
SOUTHGATE MILEPOST

- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE





— SSEP CENTERLINE SSEP LOD

FEET Danville 1:2,400 NOTES:

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200

1" = 200'

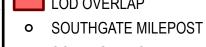
MVP Southgate Amendment Project Pittsylvania County, Virginia

FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 44 AR-022







- SOUTHGATE CENTERLINE

SOUTHGATE LOD

—— SSEP CENTERLINE

SSEP LOD

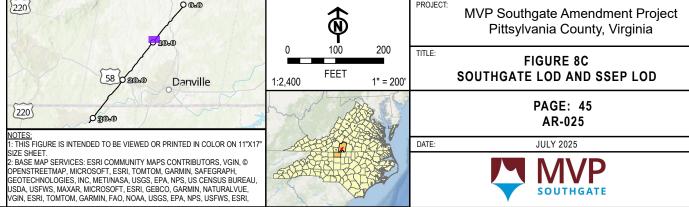




FIGURE 8C

SOUTHGATE LOD AND SSEP LOD

PAGE: 46

AR-034

JULY 2025

FEET

1:2,400

Danville

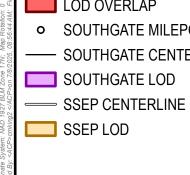
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USDA, USFWS, MAXAR, MICROSOFT, ESRI, GEBCO, GARMIN, NATURALVUE,
VGIN, ESRI, TOMTOM, GARMIN, FAO, NOAA, USGS, EPA, NPS, USFWS, ESRI,

2205

1" = 200'



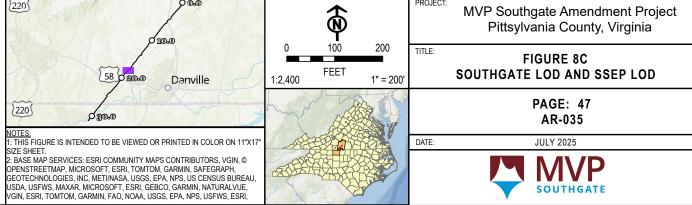
- SOUTHGATE CENTERLINE

SOUTHGATE LOD

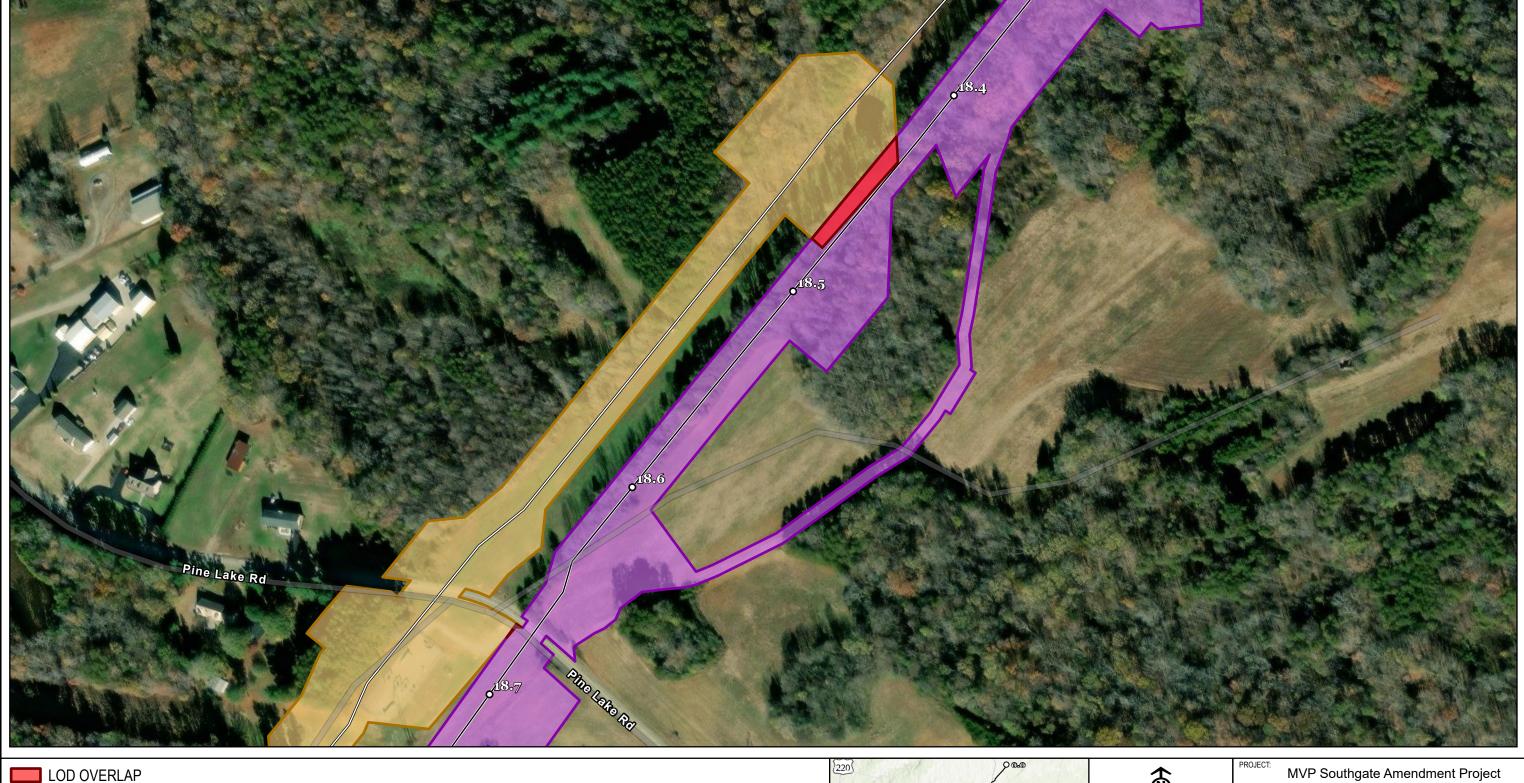
SSEP LOD

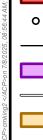






ite System: NAD 1927 BLM Zone 17N; Map Rotation: 08: 78: 2005 08: 56:44 AM





- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

SSEP LOD

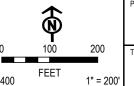




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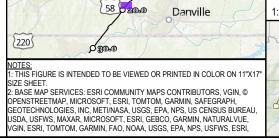
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 48 AR-037









200 1" = 200' 1:2,400

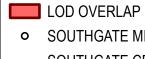
FIGURE 8C

SOUTHGATE LOD AND SSEP LOD

PAGE: 49 AR-038





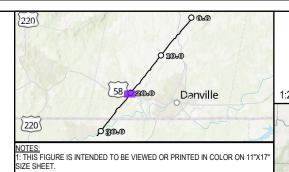


- SOUTHGATE CENTERLINE

SOUTHGATE LOD

— SSEP CENTERLINE

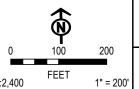
SSEP LOD



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2: BASE MAP SERVICES: ESRI COMMUNITY MAPS CONTRIBUTORS, VGIN, ©
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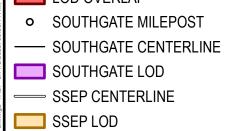
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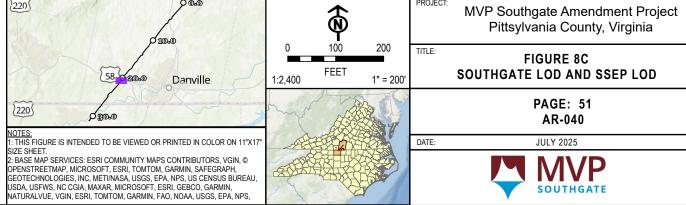
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 50 AR-039







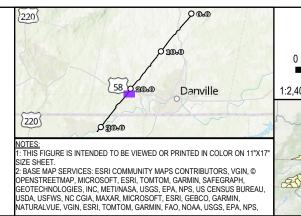


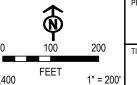
nate System: NAD 1927 BLM Zone 17N; Map Rotation: 0 d Bv. <ACP>cmkind2 </ACP>on 7/8/2025. 08:56:44 AM: Fil





— SSEP CENTERLINE SSEP LOD





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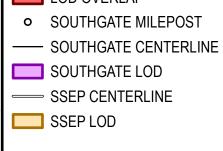
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

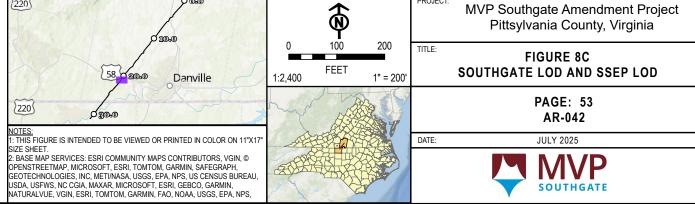
> PAGE: 52 AR-041

JULY 2025



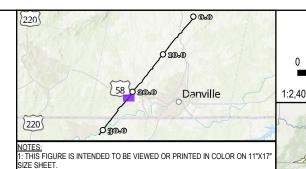








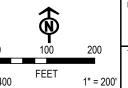




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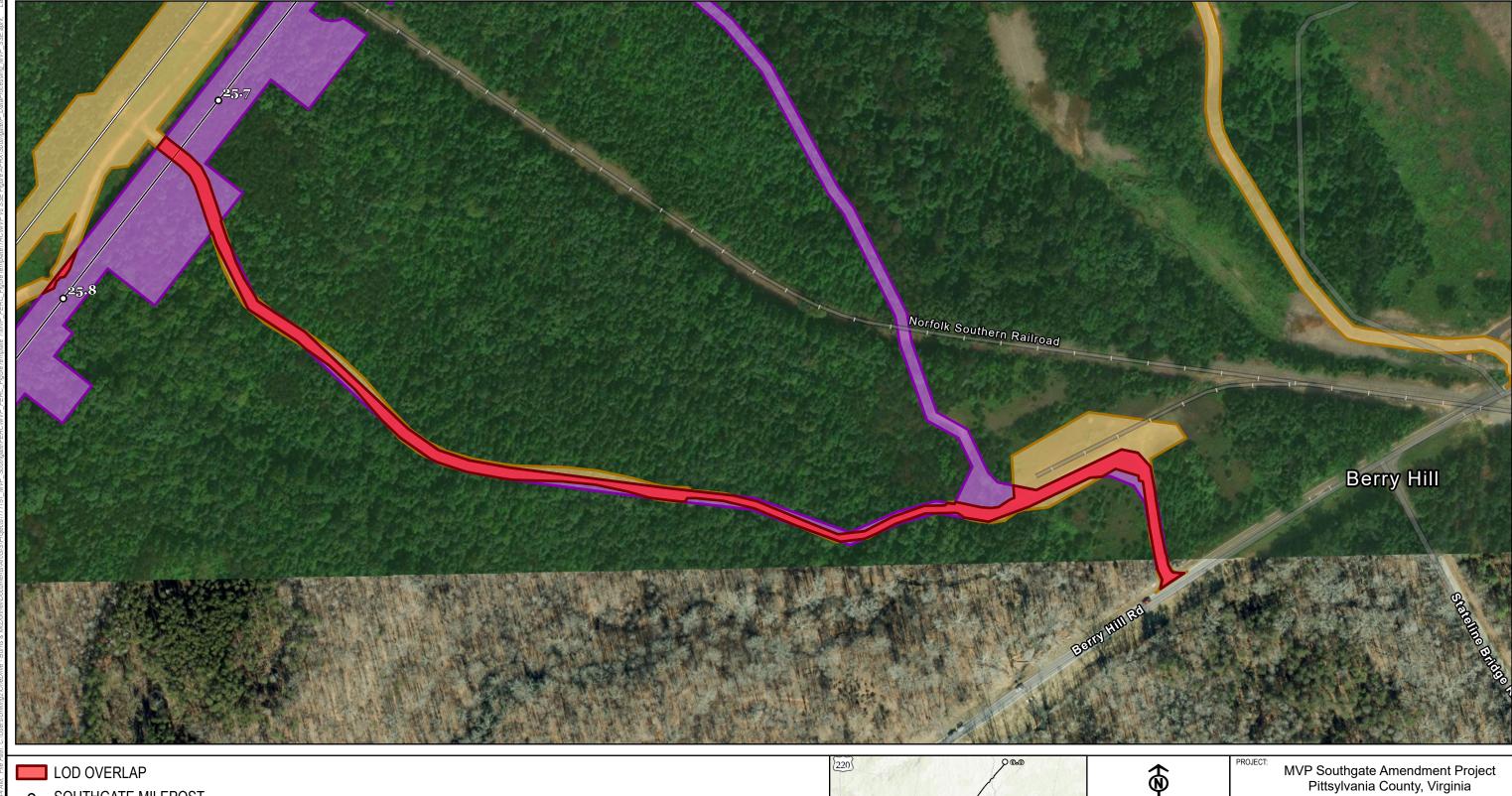
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FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 54 AR-043

JULY 2025

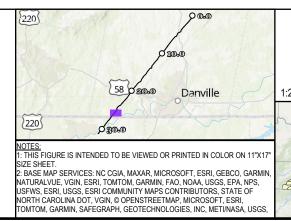






 SOUTHGATE MILEPOST - SOUTHGATE CENTERLINE SOUTHGATE LOD — SSEP CENTERLINE





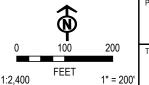


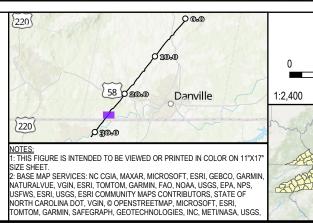
FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 55 AR-047









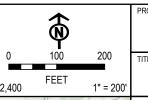
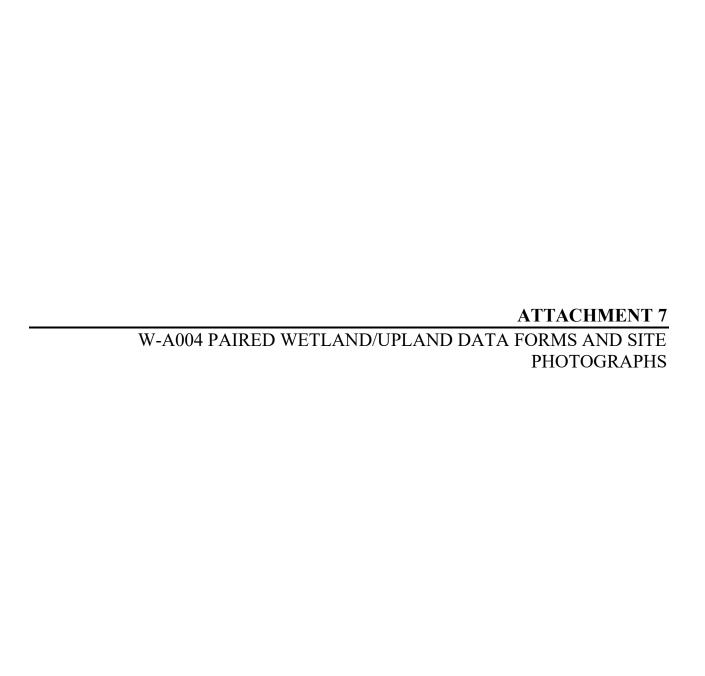


FIGURE 8C SOUTHGATE LOD AND SSEP LOD

> PAGE: 56 AR-048

JULY 2025





WETLAND DETERMINATION DATA FORM – Eastern Mountains and Piedmont Region

Project/Site: MVP Southga	ate Project		City/C	ounty: Pittsylvania (County	Sampling Date: 2024-06-04
Applicant/Owner: Mountain			-	-		Sampling Point: SP-A010
Investigator(s):W. Jackson,				on, Township, Range:		
Landform (hillslope, terrace, et						
Subregion (LRR or MLRA): P						
						ation:
Are climatic / hydrologic condit	ions on the site t	ypical for t	this time of year? Y	es No	(If no, explain in Re	emarks.)
Are Vegetation, Soil	, or Hydrolc	gy	_ significantly distur	bed? Are "Norma	al Circumstances" pi	resent? Yes No
Are Vegetation, Soil						
SUMMARY OF FINDIN	GS – Attach	site ma	p showing sam	pling point locati	ons, transects,	important features, etc.
Hydrophytic Vegetation Present? Hydric Soil Present? Wetland Hydrology Present? Remarks: Wetland sample plot wetland sample plot wetland.	Yes Yes	<u>v</u>	No No No . The USACE A	Is the Sampled Area within a Wetland?	Yes _ v	
				•		st 24 hours, hydrology
HYDROLOGY						
Wetland Hydrology Indicate	ors:				Secondary Indicat	ors (minimum of two required)
Primary Indicators (minimum		d: check a	all that apply)		Surface Soil (<u> </u>
Surface Water (A1)			rue Aquatic Plants (l	B14)		etated Concave Surface (B8)
High Water Table (A2)			ydrogen Sulfide Odo		<u>✓</u> Drainage Patt	
Saturation (A3)				es on Living Roots (C3)	_	
Water Marks (B1)		P	resence of Reduced	I Iron (C4)	Dry-Season V	Vater Table (C2)
Sediment Deposits (B2)		R	ecent Iron Reductio	n in Tilled Soils (C6)	Crayfish Burro	ows (C8)
Drift Deposits (B3)			hin Muck Surface (C			sible on Aerial Imagery (C9)
Algal Mat or Crust (B4)		0	ther (Explain in Ren	narks)		ressed Plants (D1)
Iron Deposits (B5)					Geomorphic I	
Inundation Visible on Ae					Shallow Aquit	
Water-Stained Leaves (E	39)				Microtopograp	
Aquatic Fauna (B13)					FAC-Neutral	Test (D5)
Field Observations:				;		
Surface Water Present?	Yes No	, r	Depth (inches): 0.25	,		
Water Table Present?	Yes No	, r	Depth (inches): 0			
Saturation Present? (includes capillary fringe)	Yes No	· [Depth (inches): 0	Wetland	Hydrology Present	t? Yes No
Describe Recorded Data (str	eam gauge, mon	toring we	ll, aerial photos, pre	vious inspections), if av	ailable:	
Remarks:						
	AO D10 -	1 DO				
, ,						y be misapplied due
to recent significa	nt rainfall e	event,	however, we	etland hydrolog	gy is still pre	sent.

VEGETATION (Four Strata) – Use scientific names of plants.

EGETATION (Four Strata) – Use scientific i	names of	plants.		Sampling Point: SP-A010
Total Ottatura (Dietaine 30 ft r		Dominant		Dominance Test worksheet:
Tree Stratum (Plot size: 30 ft r) 1)			·	Number of Dominant Species That Are OBL, FACW, or FAC: 2 (A)
2				Total Number of Dominant Species Across All Strata: 3 (B)
45	- -		- 	Percent of Dominant Species That Are OBL, FACW, or FAC: 66.66 (A/B)
6		r		Prevalence Index worksheet:
7		T-4-1 0		Total % Cover of: Multiply by:
50% of total cover:		= Total Cover		OBL species 0 x 1 = 0
Sapling/Shrub Stratum (Plot size: 15 ft r	20 /0 01	total cover	·	FACW species $0 x 2 = 0$
1				FAC species 10 x 3 = 30
2.				FACU species <u>5</u> x 4 = <u>20</u>
3			·	UPL species <u>0</u> x 5 = <u>0</u>
4				Column Totals: <u>15</u> (A) <u>50</u> (B)
5		·		Prevalence Index = B/A = 3.33
6				Hydrophytic Vegetation Indicators:
7				1 - Rapid Test for Hydrophytic Vegetation
8				✓ 2 - Dominance Test is >50%
9				3 - Prevalence Index is ≤3.0 ¹
		= Total Cov		4 - Morphological Adaptations ¹ (Provide supporting
50% of total cover:	20% of	total cover	·	data in Remarks or on a separate sheet)
Herb Stratum (Plot size: 5 ft r)	5	V	FAC	Problematic Hydrophytic Vegetation ¹ (Explain)
1. Microstegium vimineum 2. Persicaria longiseta	- 5		FAC	
2. Persicana longiseta 3. Parthenocissus quinquefolia	5		FACU	¹ Indicators of hydric soil and wetland hydrology must be present, unless disturbed or problematic.
4				Definitions of Four Vegetation Strata:
5				Tree Meady plants evaluding vince 2 in (7.6 cm) or
6				Tree – Woody plants, excluding vines, 3 in. (7.6 cm) or more in diameter at breast height (DBH), regardless of
7				height.
8				Sapling/Shrub – Woody plants, excluding vines, less
9				than 3 in. DBH and greater than or equal to 3.28 ft (1
10				m) tall.
11				Herb – All herbaceous (non-woody) plants, regardless
		= Total Cov		of size, and woody plants less than 3.28 ft tall.
50% of total cover: 7.5	20% of	total cover	: 3	Woody vine – All woody vines greater than 3.28 ft in
Woody Vine Stratum (Plot size: 30 ft r)				height.
1				
2				
3				
4				Hydrophytic
5				Vegetation
		= Total Cov		Present? Yes No
50% of total cover:	20% of	total cover	<u> </u>	
Remarks: (Include photo numbers here or on a separate	sheet.)			
Dominance test is met. Radii for plot	samplii	na size	s reflec	ct only areas exhibiting hydrology

Dominance test is met. Radii for plot sampling sizes reflect only areas exhibiting hydrology within the concave drainage. Trees and shrubs were all rooted outside the wetland on convex landforms.

Sampling Point: SP-A010

Profile Desc	ription: (Describe	to the de	pth needed to docur	nent the	indicator	or confirm	n the absence	of indicators.)
Depth	Matrix		Redo	x Feature	es			
(inches)	Color (moist)	%	Color (moist)	%	Type ¹	Loc ²	<u>Texture</u>	Remarks
0 - 4	10YR 6/2	100					Clay Loam	
4 - 20	10YR 6/2	80	10YR 6/6	20	С	М	Sandy Clay Loam	
	-							
			<u> </u>					
						_	-	
			<u></u> -			_	-	
-								
		_						
				-				
						_	-	
			· -					
¹Type: C=Co	oncentration, D=De	oletion, RN	/I=Reduced Matrix, M	S=Maske	d Sand G	rains.	² Location: P	L=Pore Lining, M=Matrix.
Hydric Soil		,	,					ators for Problematic Hydric Soils ³ :
Histosol	(A1)		Dark Surface	(S7)			2	cm Muck (A10) (MLRA 147)
	pipedon (A2)		Polyvalue Be		ace (S8) (I	MLRA 147		Coast Prairie Redox (A16)
Black Hi			Thin Dark Su					(MLRA 147, 148)
	n Sulfide (A4)		Loamy Gleye			,,	F	Piedmont Floodplain Soils (F19)
	d Layers (A5)		✓ Depleted Ma		(i -)		<u> </u>	(MLRA 136, 147)
	ick (A10) (LRR N)		Redox Dark		E6)		,	/ery Shallow Dark Surface (TF12)
	d Below Dark Surfac	- (Δ11)	Depleted Da	•	•			Other (Explain in Remarks)
							_ `	other (Explain in Nemarks)
	ark Surface (A12)	LDD N	Redox Depre			/I DD N		
-	Mucky Mineral (S1) (LKK N,	Iron-Mangan		ses (F12)	(LKK N,		
	A 147, 148)		MLRA 13	•	(MI DA 4)	00 400\	31	liantana af hardenda dia an aratatian and
	Gleyed Matrix (S4)		Umbric Surfa					licators of hydrophytic vegetation and
-	Redox (S5)		Piedmont Flo					etland hydrology must be present,
	Matrix (S6)		Red Parent N	Material (I	F21) (MLF	RA 127, 14	7) un	lless disturbed or problematic.
Restrictive I	Layer (if observed)	:						
Type:								
Depth (inc	ches):						Hydric Soil	Present? Yes 🗸 No
Remarks:								
In	dicator F3 is	s met.						

WETLAND DETERMINATION DATA FORM – Eastern Mountains and Piedmont Region

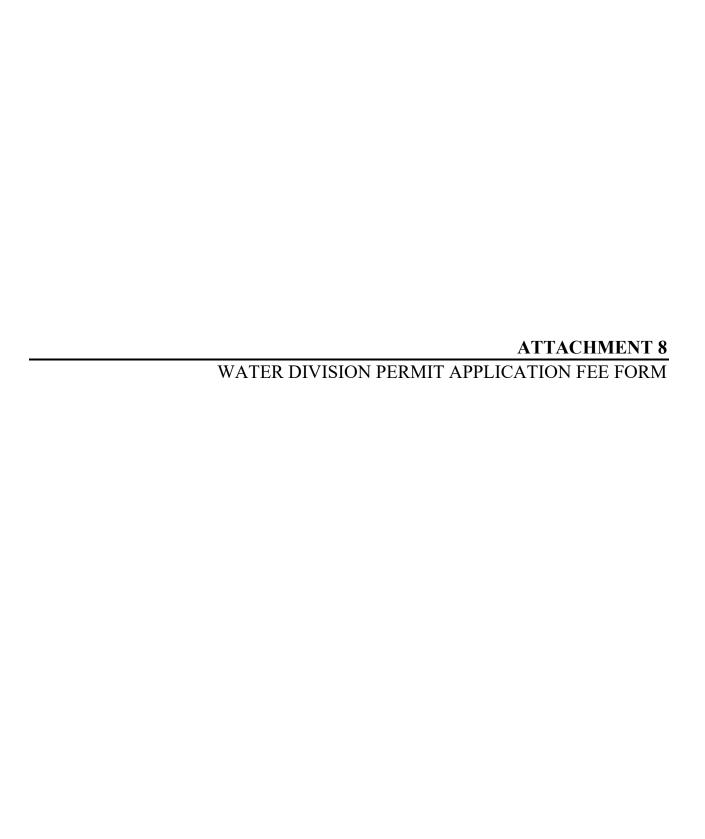
Project/Site: MVP Southgate Project City/O	County: Pittsylvania County Sampling Date: 2024-06-04
• •	State: Virginia Sampling Point: SP-A011
··· · · · · · · · · · · · · · · · · ·	on, Township, Range:
Landform (hillslope, terrace, etc.): Drainageway Local rel	
Subregion (LRR or MLRA): P 136 Lat: 36.82746	
Soil Map Unit Name: 23B - Clover fine sandy loam, 2 to 7 percei	nt slopes NWI classification:
Are climatic / hydrologic conditions on the site typical for this time of year? Y	
Are Vegetation, Soil, or Hydrology significantly distur	
Are Vegetation, Soil, or Hydrology naturally problems	
	npling point locations, transects, important features, etc.
Hydrophytic Vegetation Present? Hydric Soil Present? Wetland Hydrology Present? Yes No Yes No	Is the Sampled Area within a Wetland? Yes No
Remarks:	
Upland sample plot adjacent to PEM W-A004. The USA0 conditions were present 3 months prior to survey. Signiproblematic.	·
HYDROLOGY	
Wetland Hydrology Indicators:	Secondary Indicators (minimum of two required)
Primary Indicators (minimum of one is required; check all that apply)	Surface Soil Cracks (B6)
Surface Water (A1) True Aquatic Plants (
High Water Table (A2) Hydrogen Sulfide Od Continued Phinage has a continued	
✓ Saturation (A3) Oxidized Rhizospher Water Marks (B1) Presence of Reduced	
Water Marks (B1) Presence of Reduced Sediment Deposits (B2) Recent Iron Reduction	
Drift Deposits (B3) Thin Muck Surface (0	
Algal Mat or Crust (B4) Other (Explain in Rer	
Iron Deposits (B5)	Geomorphic Position (D2)
Inundation Visible on Aerial Imagery (B7)	Shallow Aquitard (D3)
Water-Stained Leaves (B9)	Microtopographic Relief (D4)
Aquatic Fauna (B13)	FAC-Neutral Test (D5)
Field Observations:	
Surface Water Present? Yes No Depth (inches):	
Water Table Present? Yes V No Depth (inches): 6	
Saturation Present? Yes No Depth (inches): 0	Wetland Hydrology Present? Yes No
(includes capillary fringe) Describe Recorded Data (stream gauge, monitoring well, aerial photos, pre	
, , , , ,	
Remarks:	
Indicators A2, A3, and D2 are present. Indicator	ors A2 and A3 may be misapplied due to recent
rainfall event.	

Sampling Point: SP-A011	
t worksheet:	

20.64	Absolute	Dominant		Dominance Test worksheet:	
Tree Stratum (Plot size: 30 ft r		Species?		Number of Dominant Species	
1. Acer rubrum	50		FAC	That Are OBL, FACW, or FAC: 3 (A	١)
2. Liriodendron tulipifera	15		FACU	Total Number of Dominant	
3. Quercus phellos	10		FAC	Species Across All Strata: 6 (B	3)
4. Ulmus rubra	5		FAC	Demont of Deminerat On saint	
_{5.} Juglans nigra	5		FACU	Percent of Dominant Species That Are OBL, FACW, or FAC: 50.00 (A	λ/B)
6		·			00)
7				Prevalence Index worksheet:	
	85	= Total Cov		Total % Cover of: Multiply by:	
50% of total cover: 42.5				OBL species 0 x 1 = 0	
Sapling/Shrub Stratum (Plot size: 15 ft r)				FACW species <u>5</u> x 2 = <u>10</u>	
1. Rubus allegheniensis	35	V	FACU	FAC species 135 x 3 = 405	
2. Liquidambar styraciflua	10		FAC	FACU species 90 x 4 = 360	
3. Fraxinus pennsylvanica	5		FACW	UPL species 0 x 5 = 0	
			IACVV	222	(B)
4				Column Totals (A) ((0)
5				Prevalence Index = B/A = 3.36	
6				Hydrophytic Vegetation Indicators:	
7				1 - Rapid Test for Hydrophytic Vegetation	
8				2 - Dominance Test is >50%	
9				3 - Prevalence Index is ≤3.0¹	
	50 :	= Total Cov	er		-4:
50% of total cover: 25	20% of	total cover:	10	4 - Morphological Adaptations ¹ (Provide suppor	ting
Herb Stratum (Plot size: 5 ft r)				data in Remarks or on a separate sheet)	
1. Microstegium vimineum	35	~	FAC	Problematic Hydrophytic Vegetation ¹ (Explain)	
2. Parthenocissus quinquefolia	15		FACU		
3. Liquidambar styraciflua	10		FAC	¹ Indicators of hydric soil and wetland hydrology mus	st .
4 Rubus allegheniensis	10		FACU	be present, unless disturbed or problematic.	
5. Sambucus nigra	5	-	FAC	Definitions of Four Vegetation Strata:	
6. Lonicera japonica	5	-	FACU	Tree – Woody plants, excluding vines, 3 in. (7.6 cm)) or
	5		FAC	more in diameter at breast height (DBH), regardless	
7. Euonymus americanus	5	-	FAC	height.	
8. Smilax rotundifolia	<u> </u>	-	FAC	Sapling/Shrub – Woody plants, excluding vines, les	ss
9				than 3 in. DBH and greater than or equal to 3.28 ft (
10				m) tall.	
11				Herb – All herbaceous (non-woody) plants, regardle	ess
	90 .	= Total Cov	er	of size, and woody plants less than 3.28 ft tall.	
50% of total cover: 45	20% of	total cover:	18	Woody vine – All woody vines greater than 3.28 ft i	in
Woody Vine Stratum (Plot size: 30 ft r)				height.	111
1. Parthenocissus quinquefolia	5	✓	FACU		
2					
3					
4					
5				Hydrophytic Vegetation	
<u>. </u>	5 .	= Total Cov		Present? Yes No	
50% of total cover: 2.5		total cover:	_		
Remarks: (Include photo numbers here or on a separate s					
Tremains. (include prioto numbers here or on a separate s	ilieet.)				
No tests for hydrophytic vegetation a	are met	•			

Sampling Point: SP-A011

Profile Desc	ription: (Describe	to the de	pth needed to docur	nent the	indicator	or confirm	n the absence	of indicators.)
Depth	Matrix		Redo	x Feature	es			
(inches)	Color (moist)	%	Color (moist)	%	Type ¹	Loc ²	Texture	Remarks
0 - 4	10YR 6/2	100					Clay Loam	
4 - 20	10YR 6/2	80	10YR 5/6	20	С	М	Sandy Clay Loam	
				-				
		_						
-								
		_						
				-				
		_						
¹Type: C=Co	oncentration. D=De	oletion. RN	/I=Reduced Matrix, M	S=Maske	d Sand G	rains.	² Location: P	PL=Pore Lining, M=Matrix.
Hydric Soil		, , , , , , , , , , , , , , , , , , , ,	, , , , , , , , , , , , , , , , , , , ,					ators for Problematic Hydric Soils ³ :
Histosol	(A1)		Dark Surface	e (S7)			2	2 cm Muck (A10) (MLRA 147)
	oipedon (A2)		Polyvalue Be		ace (S8) (I	MLRA 147		Coast Prairie Redox (A16)
	stic (A3)		Thin Dark Su				- -	(MLRA 147, 148)
Hydroge	en Sulfide (A4)		Loamy Gleye	ed Matrix	(F2)		F	Piedmont Floodplain Soils (F19)
Stratified	d Layers (A5)		✓ Depleted Ma	trix (F3)				(MLRA 136, 147)
2 cm Mu	ıck (A10) (LRR N)		Redox Dark	Surface (F6)		\	/ery Shallow Dark Surface (TF12)
Depleted	d Below Dark Surfac	ce (A11)	Depleted Da	rk Surface	e (F7)		0	Other (Explain in Remarks)
Thick Da	ark Surface (A12)		Redox Depre	essions (F	- 8)			
-	lucky Mineral (S1) (LRR N,	Iron-Mangan	ese Mass	ses (F12)	(LRR N,		
	A 147, 148)		MLRA 13	•				
	Gleyed Matrix (S4)		Umbric Surfa					licators of hydrophytic vegetation and
	Redox (S5)		Piedmont Flo					etland hydrology must be present,
Stripped	Matrix (S6)		Red Parent N	Material (I	=21) (MLF	RA 127, 14	7) un	lless disturbed or problematic.
Restrictive I	Layer (if observed)):						
Type:								
Depth (inc	ches):						Hydric Soil	I Present? Yes <u>✓</u> No
Remarks:								
	dicator F3 is	s met.						



DEPARTMENT OF ENVIRONMENTAL QUALITY WATER DIVISION PERMIT APPLICATION FEE FORM

INSTRUCTIONS

Applicants for individual Virginia Pollutant Discharge Elimination System (VPDES), Virginia Pollution Abatement (VPA), Virginia Water Protection (VWP), and Groundwater Withdrawal (GW) permits are required to pay permit application fees, with the exception of farming operations engaged in production for market and permits pertaining to maintenance dredging for federal navigation channels or other Corps of Engineers or Department of the Navysponsored dredging projects. Fees are also required for registration for coverage under most general permits (see Fee Schedule, page 4 of this form).

NOTE: This form is NOT appropriate for Virginia Stormwater Management Program (VSMP) Construction General Permit (VAR10) fee payments.

The permit Fee Schedule is included on pages 3-4 of this form, and includes fees for permit issuance, reissuance*, and for permit modification. Except for VWP permits, fees must be paid when applications are submitted. Applicants for VWP permits will be notified by the DEQ of the fee due. Applications will be considered incomplete if the proper fee is not paid and will not be processed until the fee is received.

* Note: the reissuance fee does not apply to individual VPDES and VPA permits - see the fee schedule for details.

Once you have determined the fee for the type of application you are submitting, complete this form. The form and your check or money order payable to "Treasurer of Virginia" should be mailed to:

Department of Environmental Quality Receipts Control P.O. Box 1104 Richmond, VA 23218

You should retain a copy of the form and your check for your records. Please direct any questions regarding this form or fee payment to the DEQ Office to which you are submitting your application.

APPLICANT NAME: Jeffrey Klinefelter
ADDRESS: 2200 Energy Drive Canonsburg PA, 15317
DAYTIME PHONE: (724) 873 -1378 IRS EMPLOYER IDENTIFICATION NUMBER (EIN): 82-5219955 [aka Federal Tax Identification Number (FIN)]
FACILITY/ACTIVITY NAME: MVP Southgate Amendment Project
LOCATION: Pittsylvania County, Virginia; Centroid of Project: 36.687423, -79.487574
TYPE OF PERMIT APPLIED FOR: VWP Individual / Surface Water Impacts (from Fee Schedule)
TYPE OF ACTION: ✓ New Issuance □ Reissuance □ Modification
AMOUNT OF FEE SUBMITTED (from Fee Schedule): \$24,180
EXISTING PERMIT NUMBER (if applicable): 25-0752
DEQ OFFICE TO WHICH APPLICATION OR REGISTRATION SUBMITTED (check one)
□ Abingdon/SWRO □ Harrisonburg/VRO □ Woodbridge/NRO □ Glen Allen/PRO ✔ Richmond/Headquarters □ Roanoke/BRRO □ Virginia Beach/TRO
¥
FOR DEQ USE ONLY
Date: DC#:

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FEE SCHEDULES

A. VPDES and VPA Individual Permits. Applications for issuance of new individual VPDES or VPA permits, and for permittee-initiated major modifications that become effective before the stated permit expiration date. (Flows listed are facility "design" flows. Land application rates listed are facility "design" rates.) [NOTE: VPDES and VPA permittees pay an Annual Permit Maintenance Fee (APMF) instead of a reapplication fee for reissuance, and the APMF is invoiced separately by DEQ.]

TYPE OF INDIVIDUAL PERMIT	ISSUANCE	MODIFICATION	LAND APP MOD
VPDES Industrial Major	\$24,000	\$12,000	
VPDES Municipal Major	\$21,300	\$10,650	\$1,000 ¹
VPDES Industrial Minor / No Standard Limits	\$10,200	\$5,100	
VPDES Industrial Minor / Standard Limits	\$3,300	\$3,300	
VPDES Industrial Stormwater	\$7,200	\$3,600	
VPDES Municipal Stormwater MS4 Individual (Large and Medium)	\$16,000	\$5,000	
VPDES Municipal Stormwater MS4 Individual (Small)	\$8,000	\$2,500	
VPDES Municipal Minor / Greater Than 100,000 GPD	\$7,500	\$3,750	\$1,000 ¹
VPDES Municipal Minor / 10,001 GPD - 100,000 GPD	\$6,000	\$3,000	\$1,000 ¹
VPDES Municipal Minor / 1,001 GPD - 10,000 GPD	\$5,400	\$2,700	\$1,000 ¹
VPDES Municipal Minor / 1,000 GPD or Less	\$2,000	\$1,000	
VPDES Municipal that includes authorization for land application, distribution, or marketing of biosolids or land disposal of sewage sludge	\$5,000 ¹	\$1,000 ¹	\$1,000 ¹
VPA Industrial Wastewater Operation / Land Application of 10 or More Inches Per Year	\$15,000	\$7,500	
VPA Industrial Wastewater Operation / Land Application of Less Than 10 Inches Per Year	\$10,500	\$5,250	
VPA Industrial Sludge Operation	\$7,500	\$3,750 ²	
VPA Municipal Wastewater Operation	\$13,500	\$6,750	
VPA Municipal Biosolids Operation	\$5,000	\$1,000 ^{2,3}	
VPA Combined Sludge Operation - Mun. Biosolids & Ind. Sludges (except WTP residuals)	\$7,500	\$3,750 ²	
All other VPA operations not specified above	\$750	\$375	

To be paid in addition to any required VPDES issuance or modification fee. The modification fee shall apply for any addition of land application sites to a permit.

B. Virginia Water Protection (VWP) Individual Permits. Applications for issuance of new individual, and reissuance or major modification of existing individual VWP permits. Only one permit application fee will be assessed per application; for a permit application involving more than one of the operations described below, the governing fee shall be based upon the primary purpose of the proposed activity. (Withdrawal amounts shown are maximum daily withdrawals.)

TYPE OF INDIVIDUAL PERMIT	ISSUANCE/REISSUANCE	MODIFICATION
VWP Individual / Surface Water Impacts (Wetlands, Streams and/or Open Water)	\$2,400 plus \$220 for each 4,356 sq. ft. (1/10 acre) (or portion thereof) of incremental impact over 87,120 sq. ft. (two acres) (\$60,000 maximum)	\$1,200 plus \$110 for each 4,356 sq. ft. (1/10 acre) (or portion thereof) of incremental impact over 87,120 sq. ft. (two acres) (\$30,000 maximum)
VWP Individual / Nonmetallic Mineral Mining	\$2,400 plus \$220 for each 4,356 sq. ft. (1/10 acre) (or portion thereof) of incremental impact over 87,120 sq. ft. (two acres) (\$7,500 maximum)	\$1,200 plus \$110 for each 4,356 sq. ft. (1/10 acre) (or portion thereof) of incremental impact over 87,120 sq. ft. (two acres) (\$3,750 maximum)
VWP Individual/Minimum Instream Flow – Surface Water Withdrawals equal to or greater than 3,000,000 gallons on any day	\$25,000	\$5,000
VWP Individual / Minimum Instream Flow – Surface Water Withdrawals between 2,000,000 and 2,999,999 gallons on any day	\$20,000	\$5,000
VWP Individual / Minimum Instream Flow – Surface Water Withdrawals between 1,000,000 and 1,999,999 gallons on any day	\$15,000	\$5,000
VWP Individual / Minimum Instream Flow – Surface Water Withdrawals < 1,000,000 gallons on any day that do not otherwise qualify for a general VWP permit for water withdrawals	\$10,000	\$5,000
VWP Individual / Reservoir – Major (any new or expanded reservoir with greater than or equal to 17 acres of total surface water impacts (stream and wetlands), or a water	\$35,000	\$12,500

² The modification fee shall apply to any addition of land application sites to a permit.

When adding any industrial source (excluding water treatment plant residuals) to a permit that only authorizes the land application of municipal biosolids, the modification fee for a VPA combined sludge operation shall apply.

TYPE OF INDIVIDUAL PERMIT	ISSUANCE/REISSUANCE	MODIFICATION
withdrawal of greater than or equal to 3,000,000 gallons in any one day)		
VWP Individual / Reservoir – Minor (any new or expanded reservoir with less than 17 acres of total surface water impacts (stream and wetlands), or a water withdrawal of less than 3,000,000 gallons in any one day)	\$25,000	\$12,500

C. Groundwater Withdrawal (GW) Individual Permits. Applications for issuance of new individual, and reissuance or major modification of existing individual GW permits.

TYPE OF INDIVIDUAL PERMIT	ISSUANCE/REISSUANCE	MODIFICATION
Groundwater Withdrawal / Initial Permit for an Existing Withdrawal Based Solely on Historic	\$1,200	\$600
Withdrawals		
Groundwater Withdrawal – effective through 12/31/2018	\$6,000	\$3,000
Groundwater Withdrawal – effective 1/1/2019	\$9,000	\$3,000

D. Registration Statements (VPDES and VPA permits) or Applications (VWP permits) for General Permit Coverage.

TYPE OF GENERAL PERMIT	ISSUANCE
VPDES General Permit for Domestic Sewage Discharges Less Than or Equal to 1,000 GPD (VAG40)	Zero (\$0)
VPDES General Permit for Discharges from Petroleum Contaminated Sites, Groundwater Remediation & Hydrostatic Tests (VAG83)	Zero (\$0)
VPDES General Permit for Discharges Resulting from the Application of Pesticides to Surface Waters of Virginia (VAG87)	Zero (\$0)
VIDEC Comment Demoit for Charma Water Discharges Associated with Industrial Astroity (VADC)	# F00
VPDES General Permit for Storm Water Discharges Associated with Industrial Activity (VAR05)	\$500
VPA General Permit for Pollutant Management Activities for Animal Feeding Operations and Animal Waste Management (VPG1)	Zero (\$0)
VPA General Permit for Poultry Waste Management (VPG2)	Zero (\$0)
VPDES General Permit for Concrete Product Facilities (VAG11)	\$600
VPDES General Permit for Noncontact Cooling Water Discharges of 50,000 GPD or Less (VAG25)	\$600
VPDES General Permit for Seafood Processing Facilities (VAG52)	\$600
VPDES General Permit for Potable Water Treatment Plants (VAG64)	\$600
VPDES General Permit for Vehicle Wash Facilities and Laundry Facilities (VAG75)	\$600
VPDES General Permit for Nonmetallic Mineral Mining (VAG84)	\$600
VPDES General Permit for Total Nitrogen and Total Phosphorus Discharges and Nutrient Trading in the Chesapeake Watershed in Virginia (VAN)	\$600
VPDES General Permit for Municipal Separate Storm Sewer Systems (MS4) – Small (VAR04)	\$4000
VWP General / Less Than 4,356 sq. ft. (1/10 acre) of Surface Water Impact (Wetlands, Streams and/or Open Water)	\$0
VWP General / 4,356 sq. ft. to 21,780 sq. ft. (1/10 acre to 1/2 acre) of Surface Water Impact (Wetlands, Streams and/or Open Water)	\$600
VWP General / 21,781 sq. ft. to 43,560 sq. ft. (greater than 1/2 acre to one acre) of Surface Water Impact (Wetlands, Streams and/or Open Water)	\$1,200
VWP General / 43,561 sq. ft. to 87,120 sq. ft. (greater than one acre to two acres) of Surface Water Impact (Wetlands, Streams and/or Open Water)	\$1,200 plus \$120 for each 4,356 sq. ft. (1/10 acre) (or portion thereof) of incremental impact over 43,560 sq. ft. (one acre) (\$2,400 maximum)
VWP General / Minimum Instream Flow / Reservoir - Water withdrawals and/or pond construction	\$2,400

NOTE: <u>This form is NOT appropriate for Virginia Stormwater Management Program (VSMP) Construction General Permit (VAR10) fee payments.</u> Please refer to the following web site hyperlink to obtain appropriate VAR10 permit fee forms:

<u>Hyperlink to the DEQ Virginia Stormwater Management, Construction Stormwater Permit Program website.</u>